

*Departamento de Ingeniería del
Terreno, Cartográfica y Geofísica*
**UNIVERSITAT POLITÈCNICA DE
CATALUNYA**



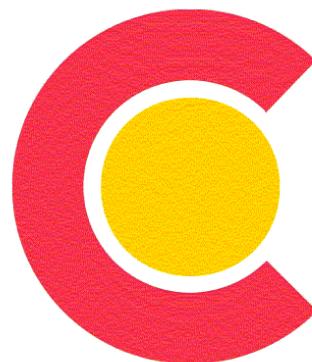
*Instituto de Investigaciones
Antisísmicas "Ing Aldo Bruschi"*
**UNIVERSIDAD NACIONAL DE
SAN JUAN**



Laboratorio de Geotecnia
**UNIVERSIDAD NACIONAL DE
CÓRDOBA**

Geotécnica e Ingeniería Sísmica aplicadas a la Minería

San Juan, Argentina, 16 de Octubre de 2007



AGENCIA
ESPAÑOLA DE
COOPERACIÓN
INTERNACIONAL





FORMULATION FOR THERMO HYDRO MECHANICAL PROBLEMS IN POROUS MEDIA

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San Juan, 16 de octubre 2007

Índice

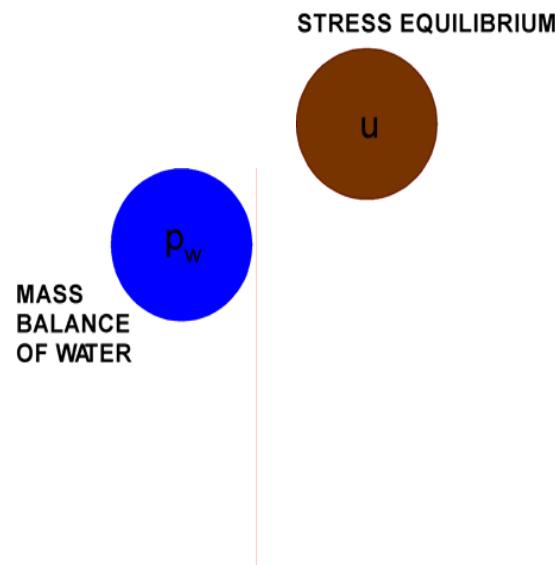
1. PROBLEMAS ACOPLADOS
2. FORMULACION BASICA
3. ALGUNOS EJEMPLOS

PROBLEMAS ACOPLADOS



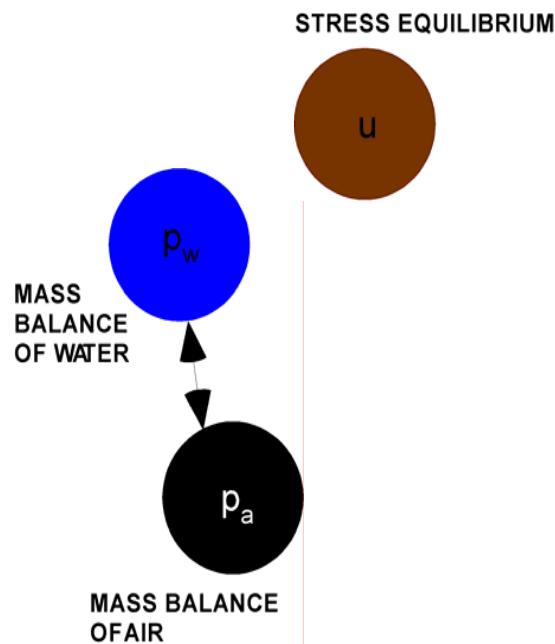
Mechanical problems
in soils and rocks

PROBLEMAS ACOPLADOS



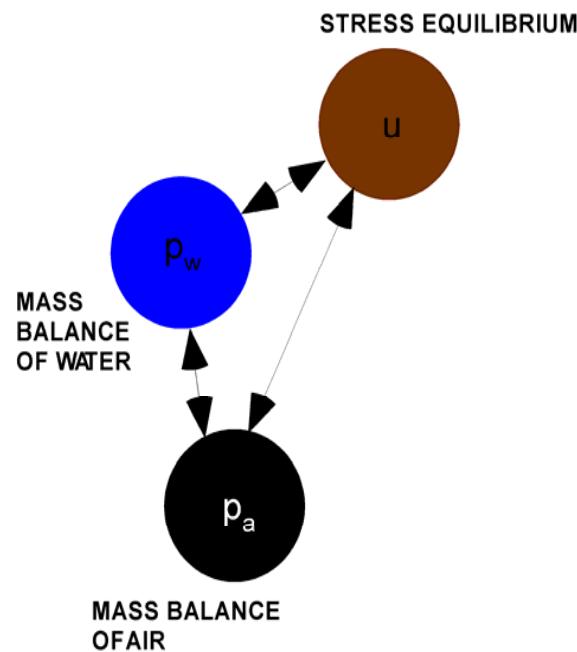
Mechanical problems
in soils and rocks
One phase flow

PROBLEMAS ACOPLADOS



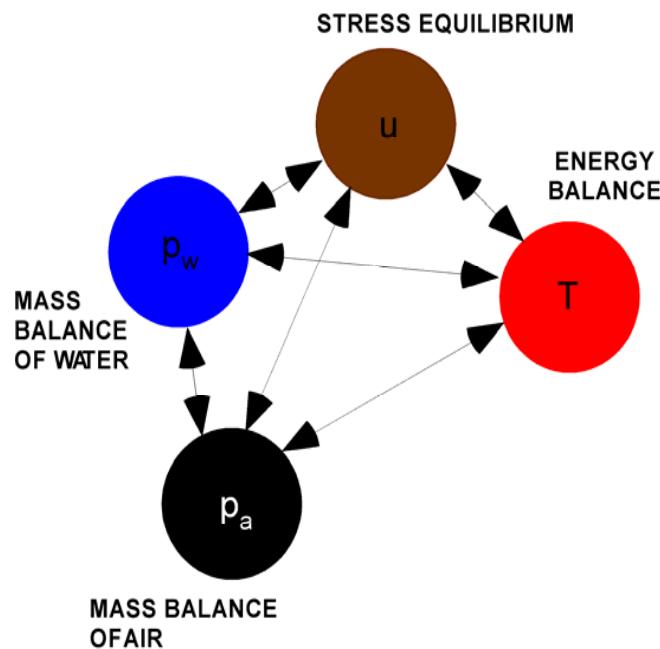
Mechanical problems
in soils and rocks
One phase flow
Two phase flow

PROBLEMAS ACOPLADOS



Mechanical problems
in soils and rocks
One phase flow
Two phase flow
Saturated consolidation/
unsaturated consolidation

PROBLEMAS ACOPLADOS



Mechanical problems
in soils and rocks

One phase flow

Two phase flow

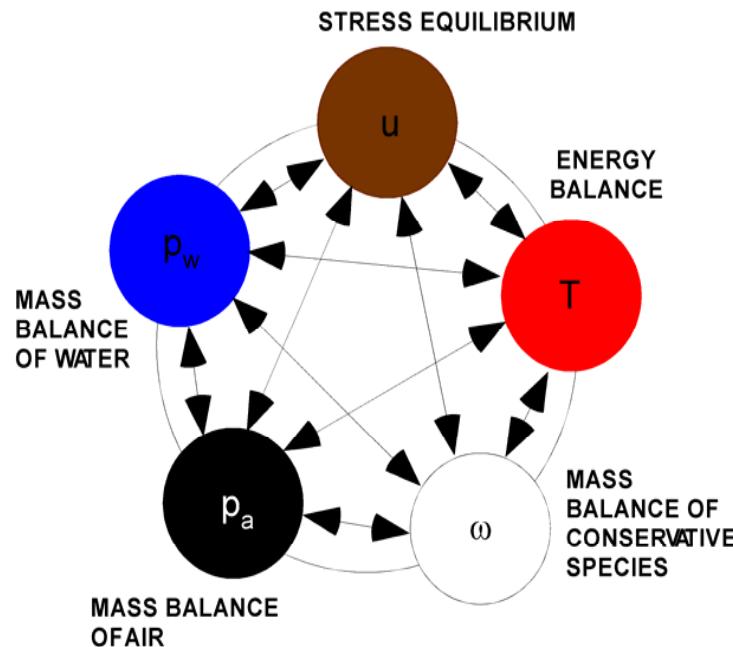
**Saturated consolidation/
unsaturated consolidation**

Evaporation/evapotranspiration

Waste disposals

Geothermal fluid extraction

PROBLEMAS ACOPLADOS



Mechanical problems
in soils and rocks

One phase flow

Two phase flow

**Saturated consolidation/
unsaturated consolidation**

Evaporation/evapotranspiration

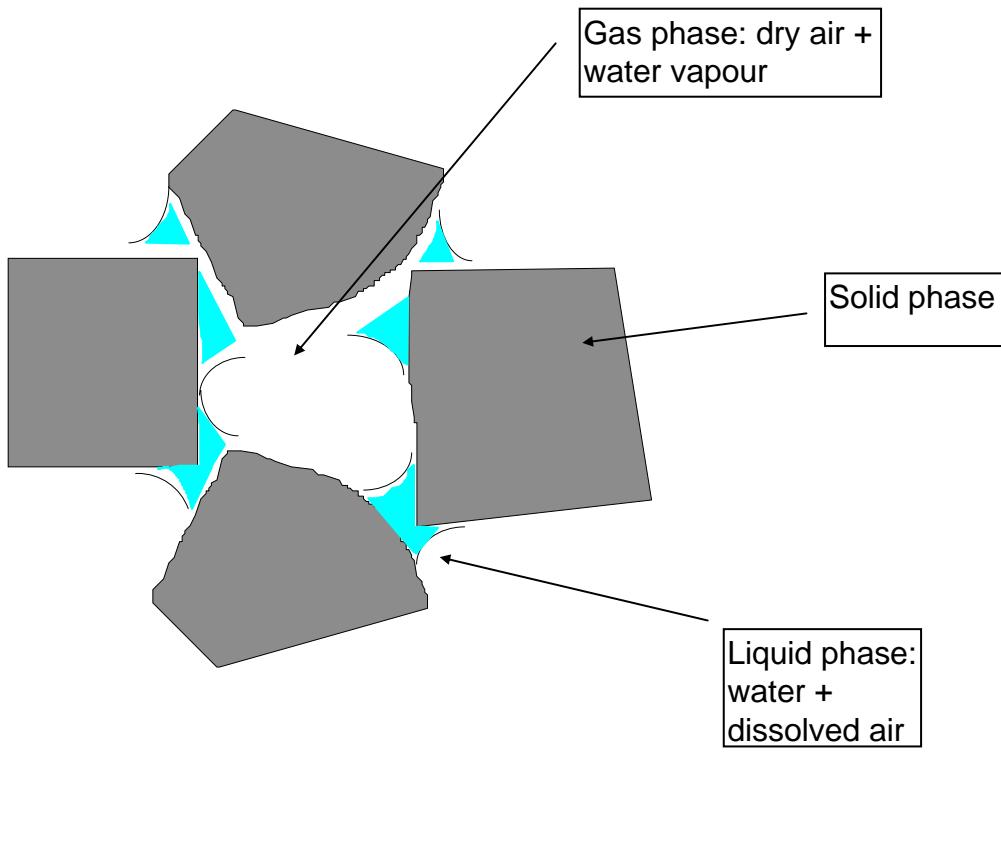
Waste disposals

Geothermal fluid extraction

**Solute transport
problems**

CODE_BRIGHT

2. BASIC FORMULATION



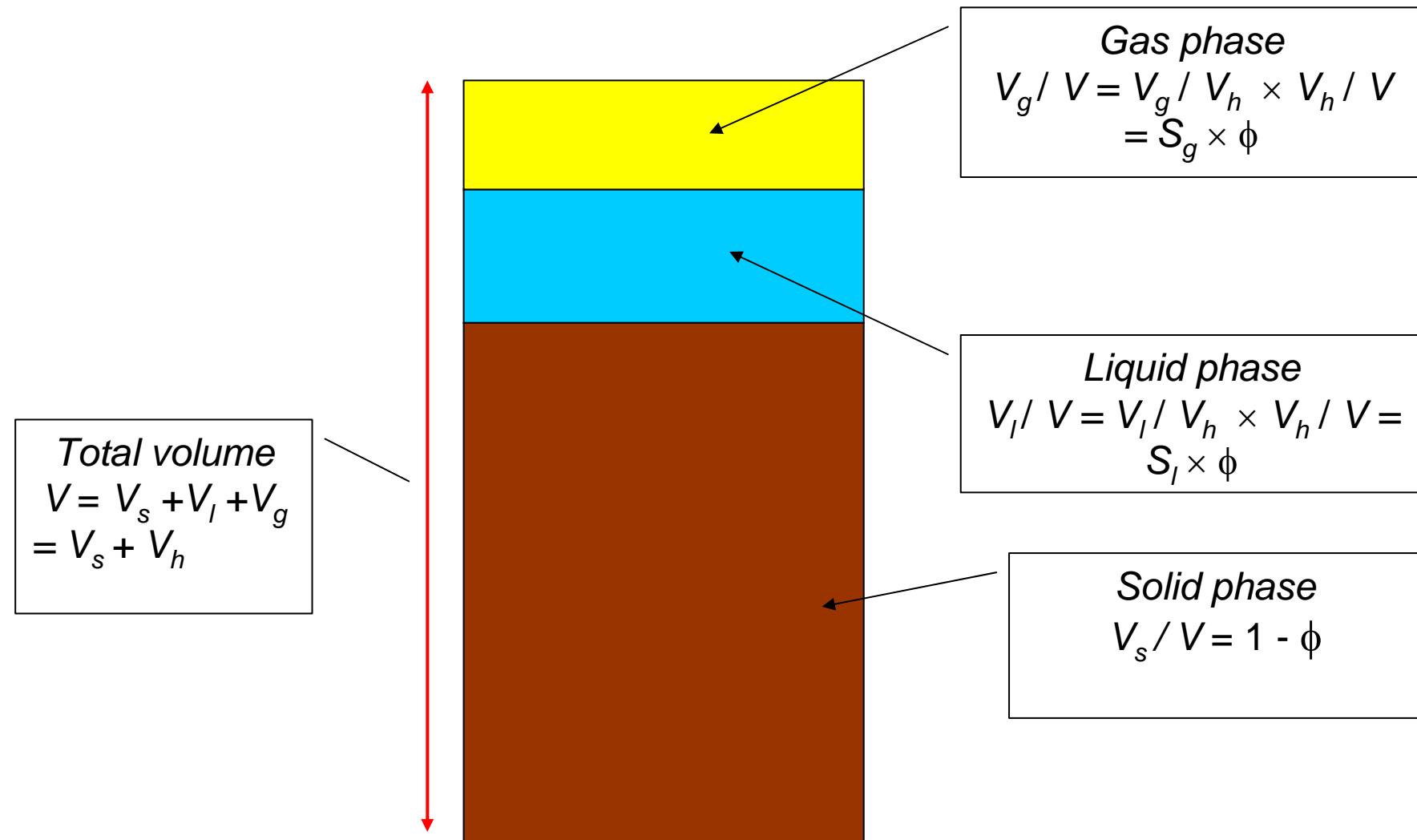
The three phases are:

- solid phase (s) :mineral
- liquid phase (*l*) :water + air dissolved
- gas phase (g) :mixture of dry air and water vapour

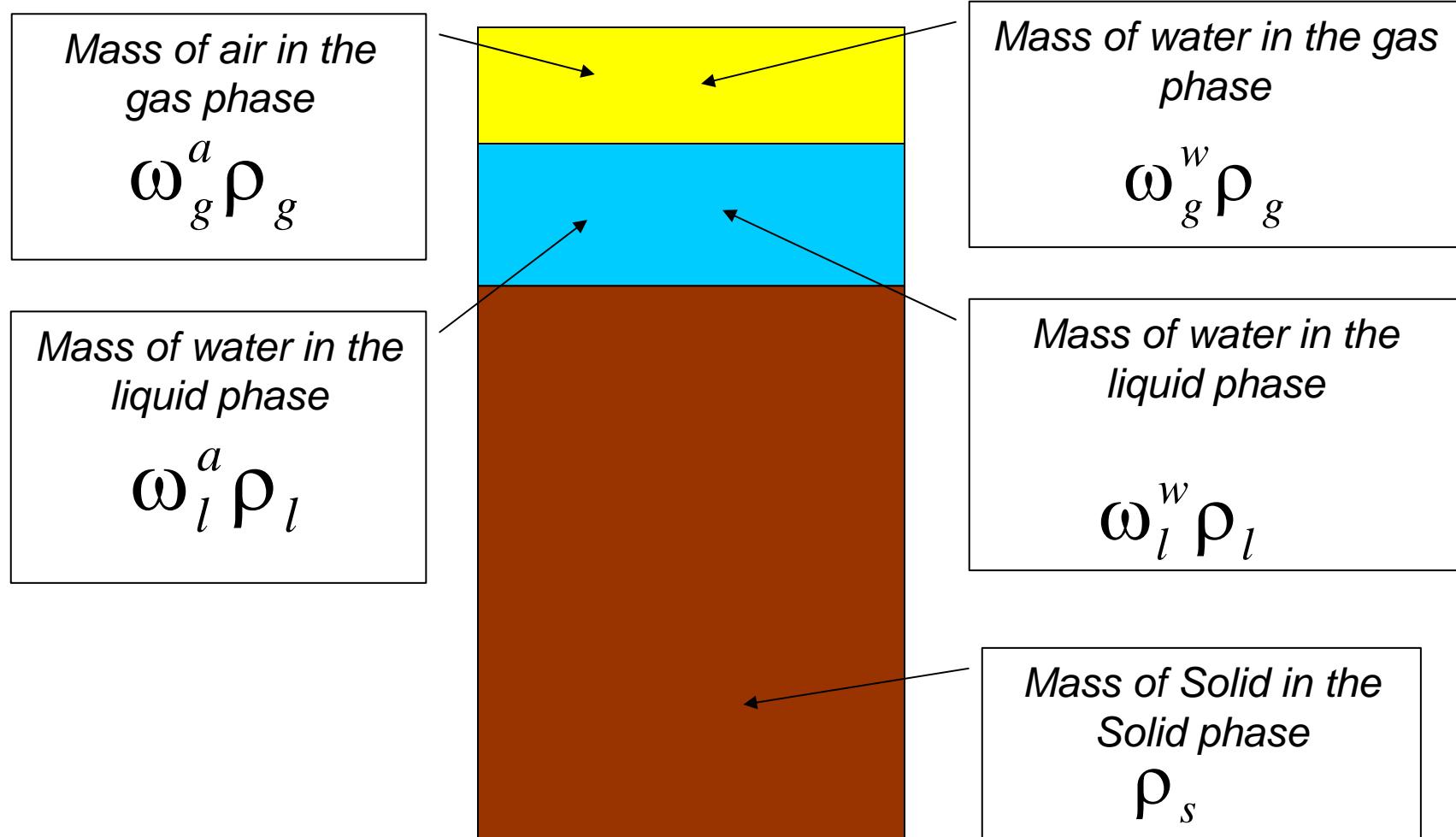
The three species are:

- Solid (-): the mineral is coincident with solid phase
- Water (w):as liquid or evaporated in the gas phase
- Air (a) :dry air, as gas or dissolved in the liquid phase

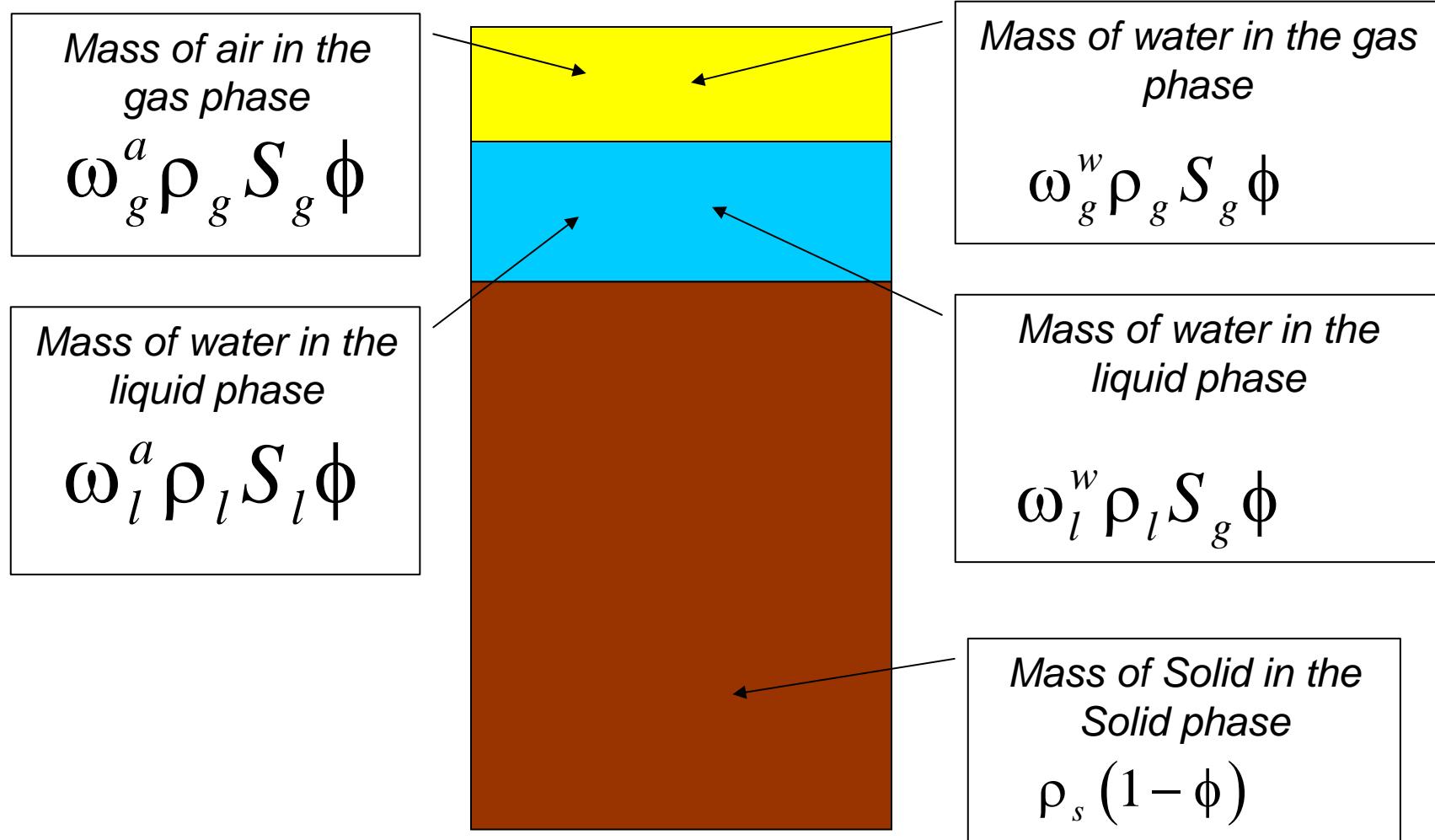
Unsaturated soil: Porosity and degree of Saturation

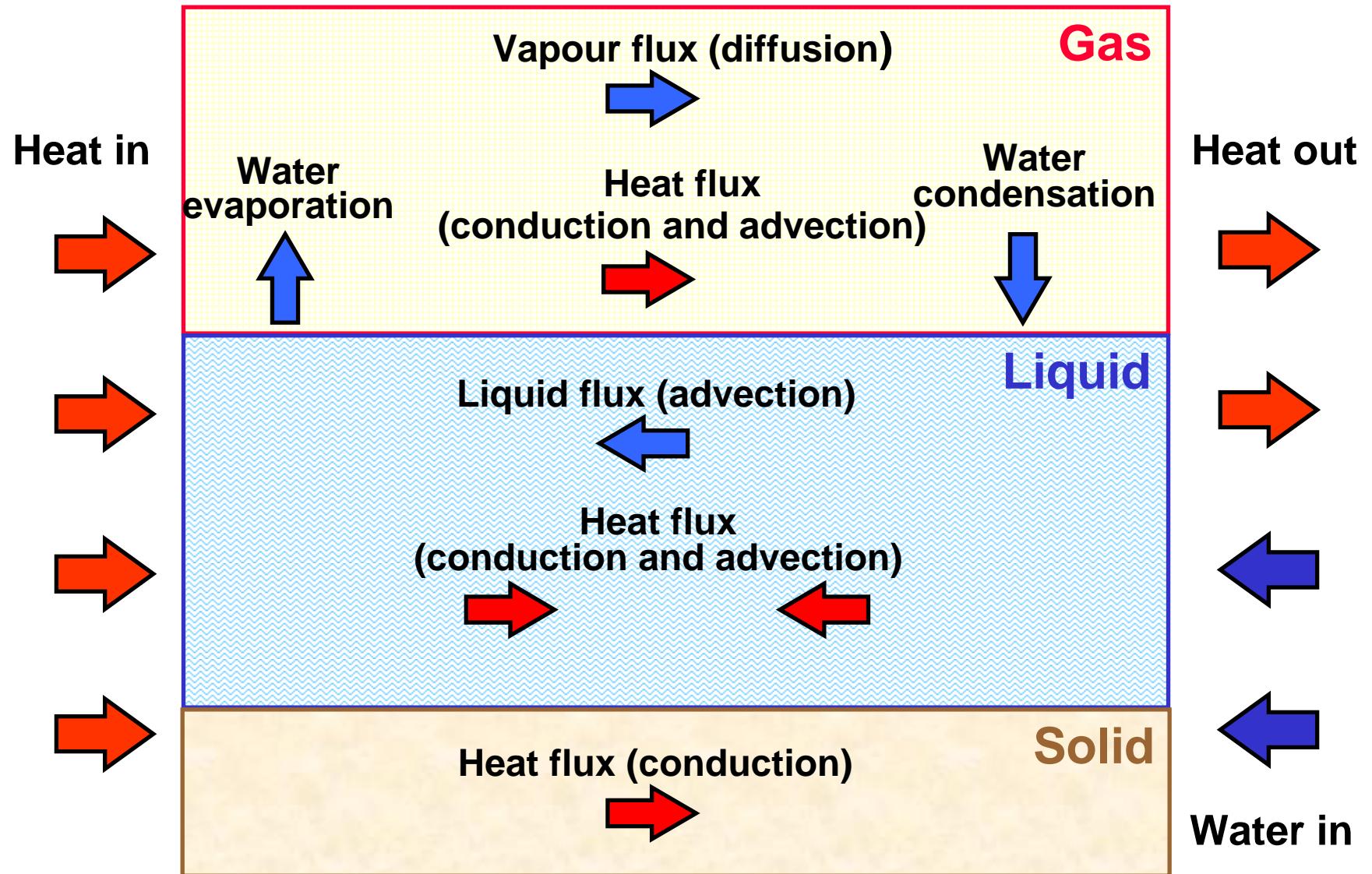


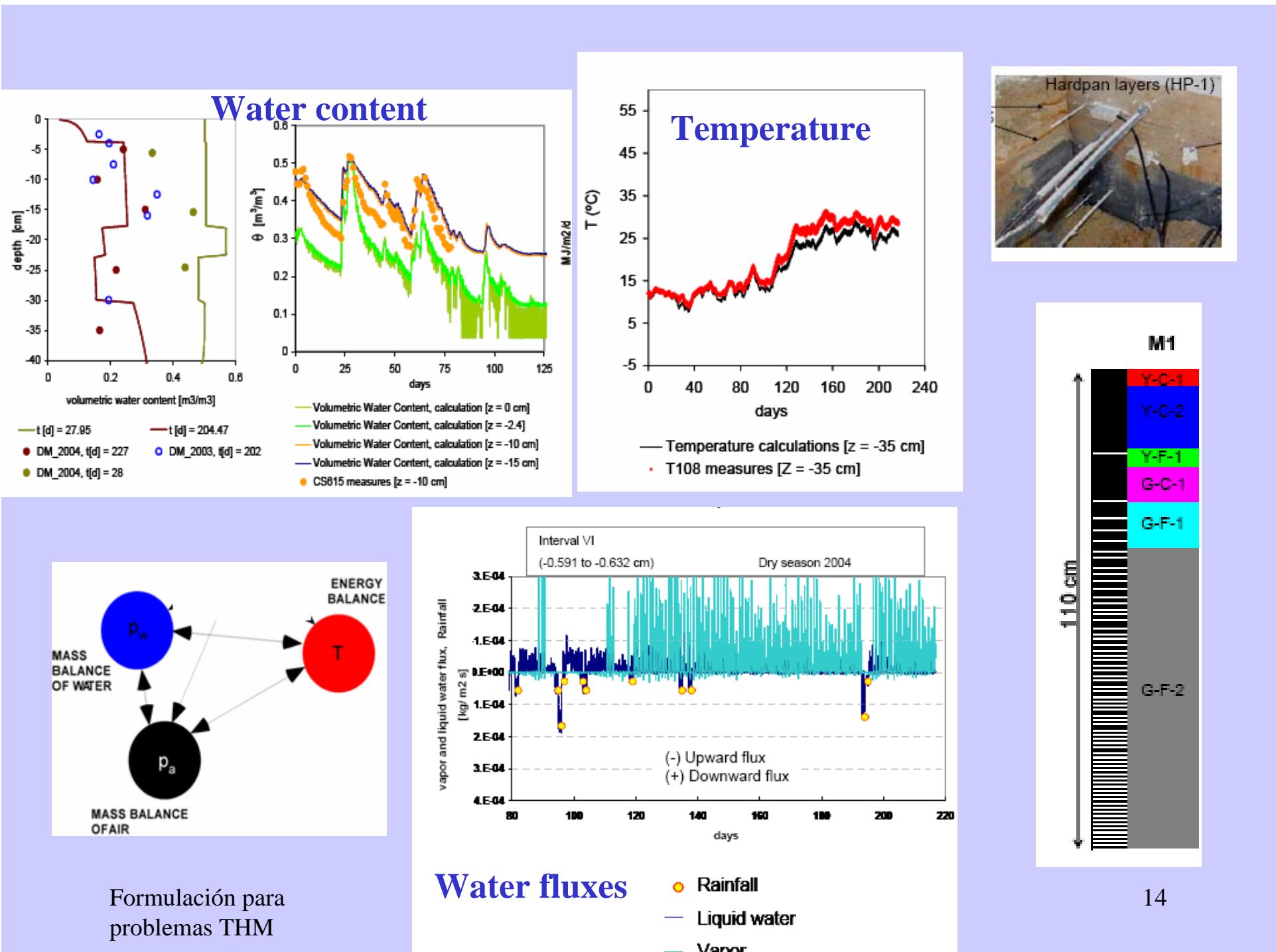
Unsaturated soil: Mass in phases



Unsaturated soil: Mass in porous medium

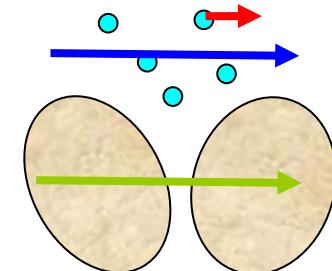






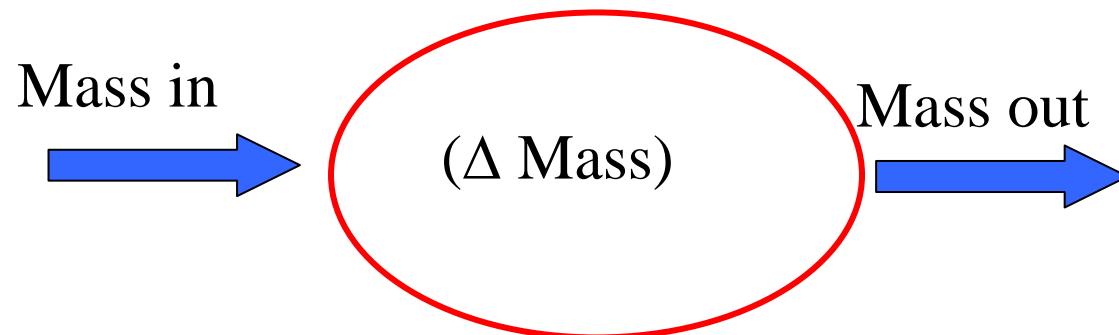
THE TOTAL MASS FLUX OF A SPECIES IN A PHASE (E.G. FLUX OF AIR PRESENT IN GAS PHASE j_g^w) IS, IN GENERAL, THE SUM OF THREE TERMS

the nonadvection flux: \mathbf{i}_g^w , i.e. diffusive/ dispersive,	\mathbf{i}_g^w
the advective flux caused by fluid motion: $\omega_g^w \rho_g \mathbf{q}_g$, where \mathbf{q}_g is the Darcy's flux,	$\omega_g^w \rho_g \mathbf{q}_g$
the advective flux caused by solid motion: $\phi S_g \omega_g^w \rho_g d\mathbf{u}/dt$ where $d\mathbf{u}/dt$ is the vector of solid velocities, S_g is the volumetric fraction of pores occupied by the gas phase and ϕ is porosity.	$\phi S_g \omega_g^w \rho_g d\mathbf{u}/dt$
	$\phi S_g \omega_g^w \rho_g d\mathbf{u}/dt$ $\omega_g^w \rho_g \mathbf{q}_g$ \mathbf{i}_g^w



The following balance equation will be applied

$$\frac{\partial}{\partial t} \left(\begin{array}{c} \text{mass or energy per} \\ \text{unit volume of porous media} \end{array} \right) + \nabla \cdot \left(\begin{array}{c} \text{fluxes of mass} \\ \text{or energy} \end{array} \right) = \\ = \left(\begin{array}{c} \text{sources or sinks of} \\ \text{mass and energy} \end{array} \right)$$



MASS BALANCE OF SOLID

Mass balance of solid:

$$\frac{\partial}{\partial t} \left(\rho_s (1 - \phi) \right) + \nabla \cdot (\mathbf{j}_s) = 0$$

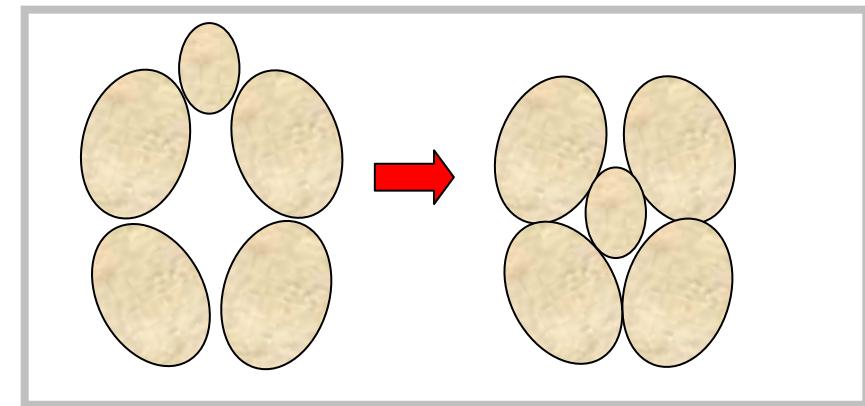
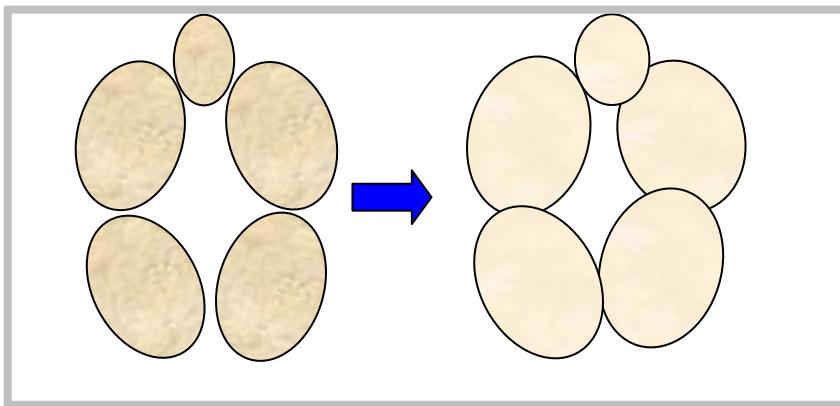
ρ_s solid density and \mathbf{j}_s is the flux of solid.

$$\frac{\partial}{\partial t} \left(\rho_s (1 - \phi) \right) + \nabla \cdot \left(\rho_s (1 - \phi) \frac{d\mathbf{u}}{dt} \right) = 0$$

Porosity variation:

$$\frac{D_s \phi}{Dt} = \frac{1}{\rho_s} \left[(1 - \phi) \frac{D_s \rho_s}{Dt} \right] + (1 - \phi) \nabla \cdot \frac{d\mathbf{u}}{dt}$$

$$\frac{D_s \phi}{Dt} = \frac{1}{\rho_s} \left[(1-\phi) \frac{D_s \rho_s}{Dt} \right] + (1-\phi) \nabla \cdot \frac{d\mathbf{u}}{dt}$$

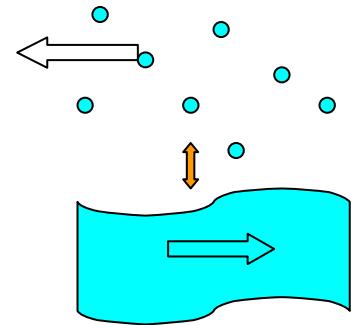


Volumetric strain:

$$\nabla \cdot \frac{d\mathbf{u}}{dt} = \frac{d\varepsilon_x}{dt} + \frac{d\varepsilon_y}{dt} + \frac{d\varepsilon_z}{dt}$$

MASS BALANCE OF WATER

Water is present in liquid and gas phases. The mass balance of water in the liquid and in the gas phase is expressed as:



$$\frac{\partial}{\partial t} (\omega_g^w \rho_g S_g \phi) + \nabla \cdot (\mathbf{j}_g^w) = f^{evaporation} + f^w$$
$$\frac{\partial}{\partial t} (\omega_l^w \rho_l S_l \phi) + \nabla \cdot (\mathbf{j}_l^w) = f^{condensation} + f^w$$

f^w is an external supply of water and:

$$f^{condensation} + f^{evaporation} = 0$$

Adding the two mass balance equations leads to:

$$\frac{\partial}{\partial t} (\omega_l^w \rho_l S_l \phi + \omega_g^w \rho_g S_g \phi) + \nabla \cdot (\mathbf{j}_l^w + \mathbf{j}_g^w) = f^w$$

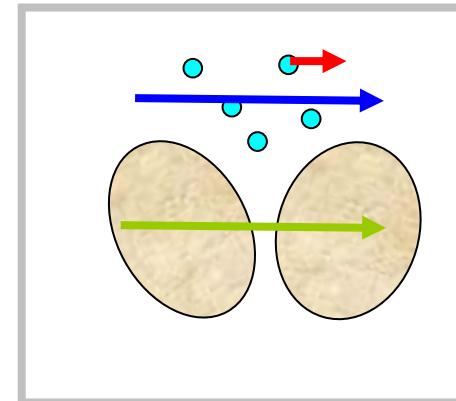
$$\frac{\partial}{\partial t} \left(\omega_l^w \rho_l S_l \phi + \omega_g^w \rho_g S_g \phi \right) + \nabla \cdot \left(\mathbf{j}_l^w + \mathbf{j}_g^w \right) = f^w$$

Flux referred to a fixed framework:

$$\mathbf{j}_g^w = \mathbf{i}_g^w + \omega_g^w \rho_g \mathbf{q}_g + \phi S_g \omega_g^w \rho_g du/dt = \mathbf{j}'_g^w + \phi S_g \omega_g^w \rho_g du/dt$$


Flux referred to the solid skeleton:

$$\mathbf{j}'_g^w = \mathbf{i}_g^w + \omega_g^w \rho_g \mathbf{q}_g$$



The use of the material derivative leads to:

$$\begin{aligned} & \phi \frac{D_s (\omega_l^w \rho_l S_l + \omega_g^w \rho_g S_g)}{Dt} + (\omega_l^w \rho_l S_l + \omega_g^w \rho_g S_g) \frac{D_s \phi}{Dt} + \\ & + ((\omega_l^w \rho_l S_l + \omega_g^w \rho_g S_g) \phi) \nabla \cdot \frac{d\mathbf{u}}{dt} + \nabla \cdot (\mathbf{j}_l^{'w} + \mathbf{j}_g^{'w}) = f^w \end{aligned}$$

Porosity appears in this equation as:

- a coefficient in storage terms
- in a term involving its variation caused by different processes
- hidden in variables that depend on porosity (e.g. intrinsic permeability).

Porosity from solid balance leads to:

$$\phi \frac{D_s (\omega_l^w \rho_l S_l + \omega_g^w \rho_g S_g)}{Dt} + (\omega_l^w \rho_l S_l + \omega_g^w \rho_g S_g) \frac{1 - \phi}{\rho_s} \frac{D_s \rho_s}{Dt} +$$

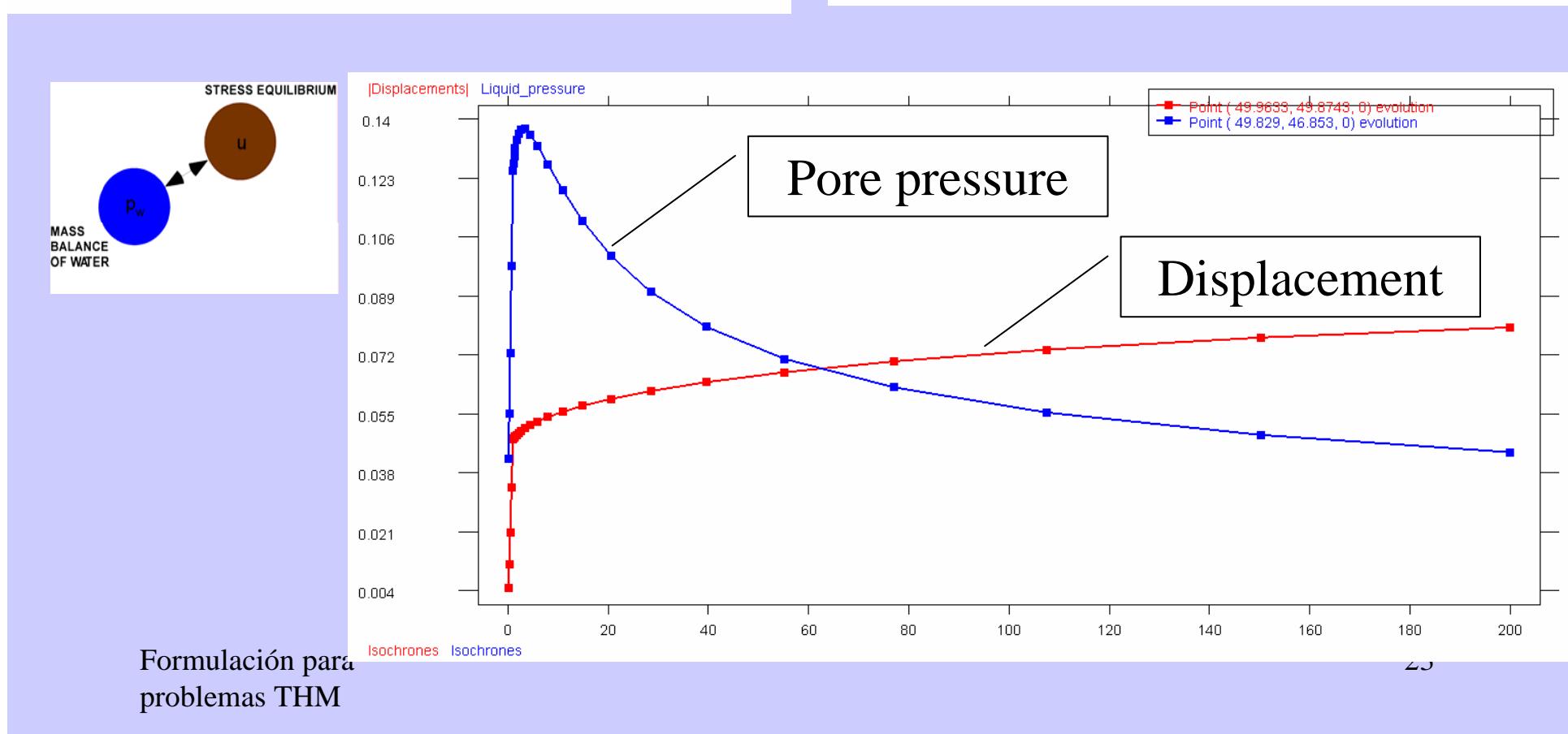
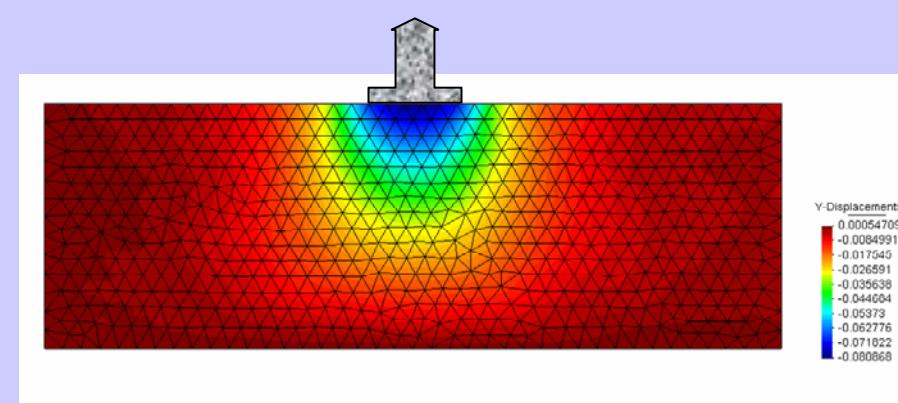
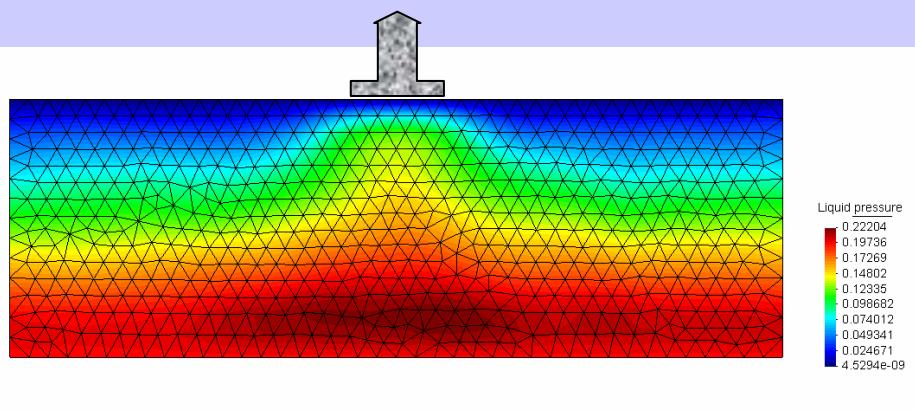
$$+ \left((\omega_l^w \rho_l S_l + \omega_g^w \rho_g S_g) \right) \nabla \cdot \frac{d\mathbf{u}}{dt} + \nabla \cdot (\mathbf{j}_l^w + \mathbf{j}_g^w) = f^w$$

For incompressible solid:

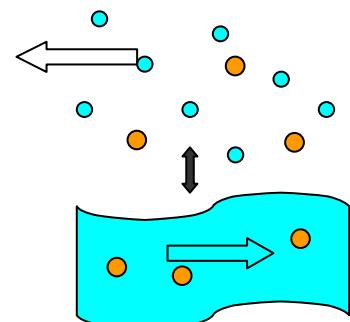
$$\phi \frac{D_s (\omega_l^w \rho_l S_l + \omega_g^w \rho_g S_g)}{Dt} + \nabla \cdot (\mathbf{j}_l^w + \mathbf{j}_g^w) =$$

$$= - \left((\omega_l^w \rho_l S_l + \omega_g^w \rho_g S_g) \right) \nabla \cdot \frac{d\mathbf{u}}{dt} + f^w$$

Foundation in a saturated soil



MASS BALANCE OF AIR



$$\frac{\partial}{\partial t} (\omega_g^a \rho_g S_g \phi) + \nabla \cdot (\mathbf{j}_g^a) = f^{liberation} + f^a$$

$$\frac{\partial}{\partial t} (\omega_l^a \rho_l S_l \phi) + \nabla \cdot (\mathbf{j}_l^a) = f^{dissolution} + f^a$$

where f^a is an external supply of air and the internal source terms that represent phase change are:

$$f^{dissolution} + f^{liberation} = 0$$

Adding the two equations leads to:

$$\frac{\partial}{\partial t} (\omega_l^a \rho_l S_l \phi + \omega_g^a \rho_g S_g \phi) + \nabla \cdot (\mathbf{j}_l^a + \mathbf{j}_g^a) = f^a$$

An internal production term is not present because the total mass balance inside the medium is considered in this equation.

MOMENTUM BALANCE FOR THE MEDIUM

Momentum balance = equilibrium of stresses
(inertial terms neglected)

$$\nabla \cdot \sigma + \mathbf{b} = 0$$

where σ is the stress tensor and \mathbf{b} is the vector of body forces.

INTERNAL ENERGY BALANCE FOR THE MEDIUM

Internal energy balance for the porous medium (e_s , e_l , e_g):

$$\frac{\partial}{\partial t} \left(e_s \rho_s (1 - \phi) + e_l \rho_l S_l \phi + e_g \rho_g S_g \phi \right) + \nabla \cdot (\mathbf{i}_c + \mathbf{j}_{es} + \mathbf{j}_{el} + \mathbf{j}_{eg}) = f^Q$$

- \mathbf{i}_c is energy flux: conduction through the porous medium
- $(\mathbf{j}_{es}, \mathbf{j}_{el}, \mathbf{j}_{eg})$ are advective fluxes of energy caused by mass motions
- f^Q is an internal/external energy supply.

Advective heat fluxes due to mass movement

The internal energy in each phase can be calculated as a function of internal energy of each component.

$$e_g \rho_g = e_g^w \omega_g^w \rho_g + e_g^a \omega_g^a \rho_g = (e_g^w \omega_g^w + e_g^a \omega_g^a) \rho_g$$

$$e_g = e_g^w \omega_g^w + e_g^a \omega_g^a$$

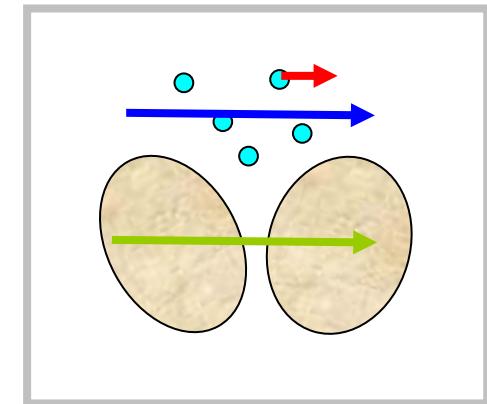
Following this decomposition, the advective fluxes of energy in each phase are calculated as:

$$\mathbf{j}_g = e_g^w \mathbf{j}_g^w + e_g^a \mathbf{j}_g^a$$

$$e_g^w \mathbf{j}_g^w = e_g^w \left(\mathbf{i}_g^w + \boldsymbol{\omega}_g^w \rho_g \mathbf{q}_g + \boldsymbol{\omega}_g^w \rho_g S_g \phi \frac{d\mathbf{u}}{dt} \right)$$

$$e_g^a \mathbf{j}_g^a = e_g^a \left(\mathbf{i}_g^a + \boldsymbol{\omega}_g^a \rho_g \mathbf{q}_g + \boldsymbol{\omega}_g^a \rho_g S_g \phi \frac{d\mathbf{u}}{dt} \right)$$

$$\mathbf{j}_g = e_g^w \mathbf{j}_g^w + e_g^a \mathbf{j}_g^a = e_g^w \mathbf{i}_g^w + e_g^a \mathbf{i}_g^a + e_g \rho_g \left(\mathbf{q}_g + S_g \phi \frac{d\mathbf{u}}{dt} \right)$$



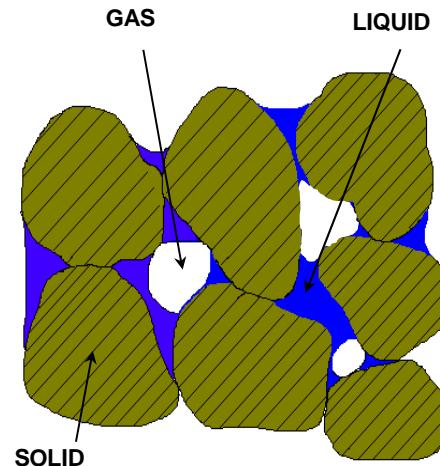
FINAL SET OF BALANCE EQUATIONS

MASS BALANCE OF WATER. Unknown: P_l

$$\frac{\partial}{\partial t} (\omega_l^w \rho_l S_l \phi + \omega_g^w \rho_g S_g \phi) + \nabla \cdot (\mathbf{j}_l^w + \mathbf{j}_g^w) = f^w$$

MASS BALANCE OF AIR. Unknown: P_g

$$\frac{\partial}{\partial t} (\omega_l^a \rho_l S_l \phi + \omega_g^a \rho_g S_g \phi) + \nabla \cdot (\mathbf{j}_l^a + \mathbf{j}_g^a) = f^a$$



INTERNAL ENERGY BALANCE FOR THE MEDIUM. Unknown: T

$$\frac{\partial}{\partial t} (E_s \rho_s (1-\phi) + E_l \rho_l S_l \phi + E_g \rho_g S_g \phi) + \nabla \cdot (\mathbf{i}_c + \mathbf{j}_{Es} + \mathbf{j}_{El} + \mathbf{j}_{Eg}) = f^Q$$

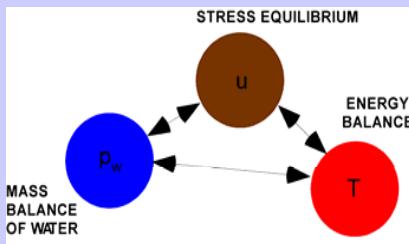
MOMENTUM BALANCE FOR THE MEDIUM. Unknown: \mathbf{u}

$$\nabla \cdot \boldsymbol{\sigma} + \mathbf{b} = \mathbf{0}$$

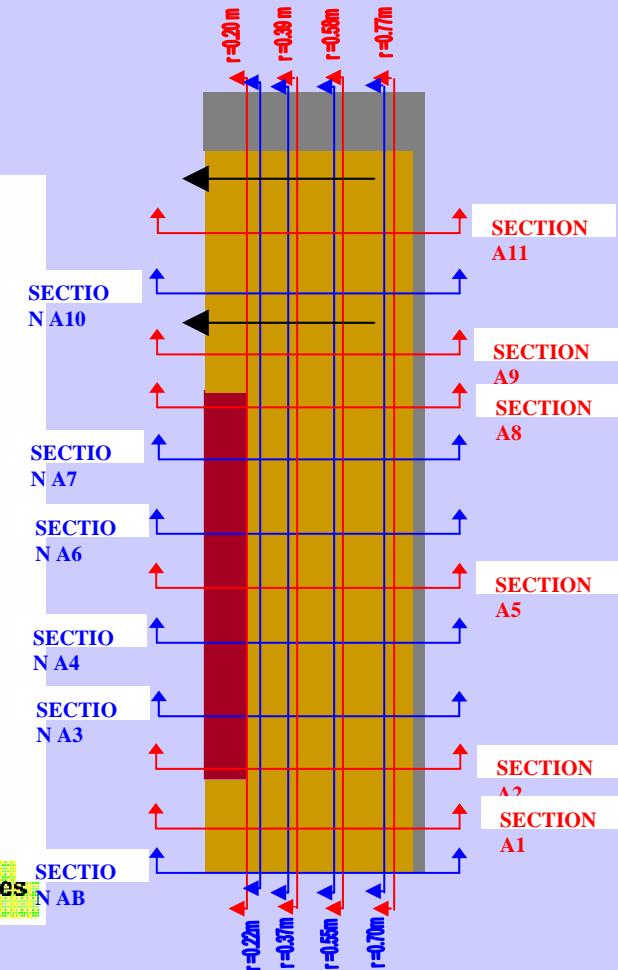
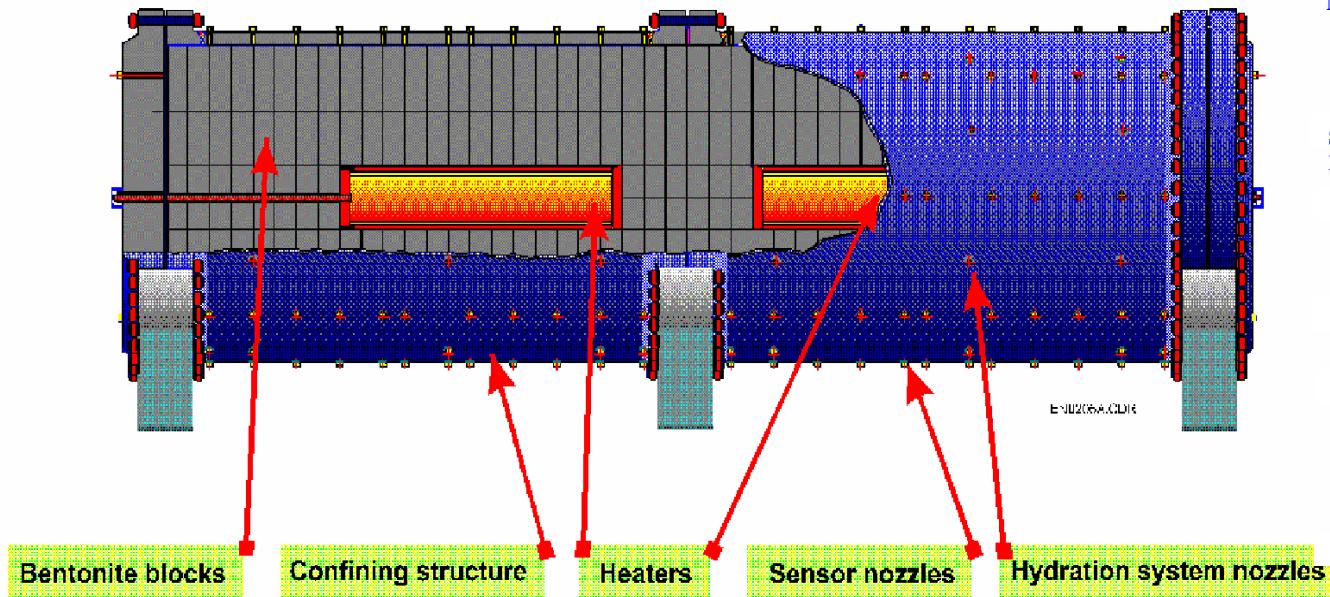
SPECIES MASS BALANCE (REACTIVE TRANSPORT). Unknown: \mathbf{c}

$$\frac{\partial}{\partial t} (\phi S_l \rho_l c) + \nabla \cdot \mathbf{j} = R$$

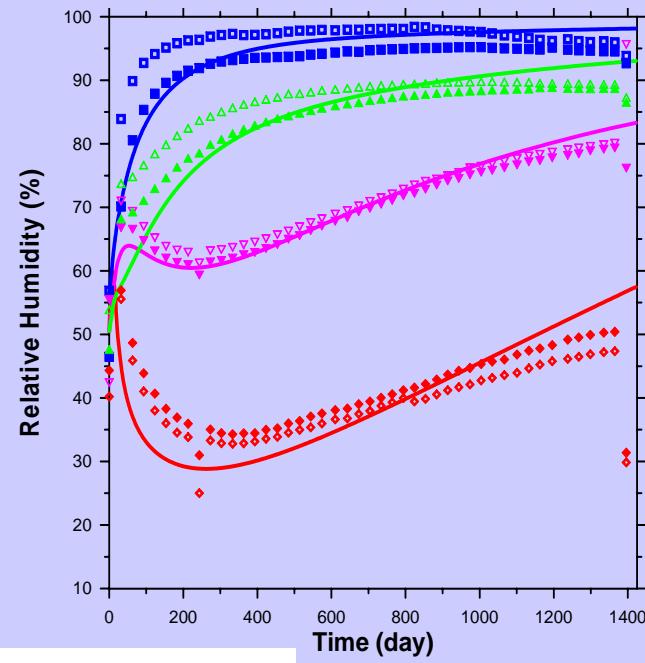
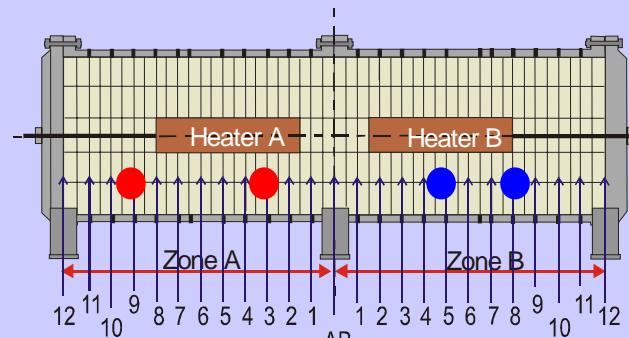
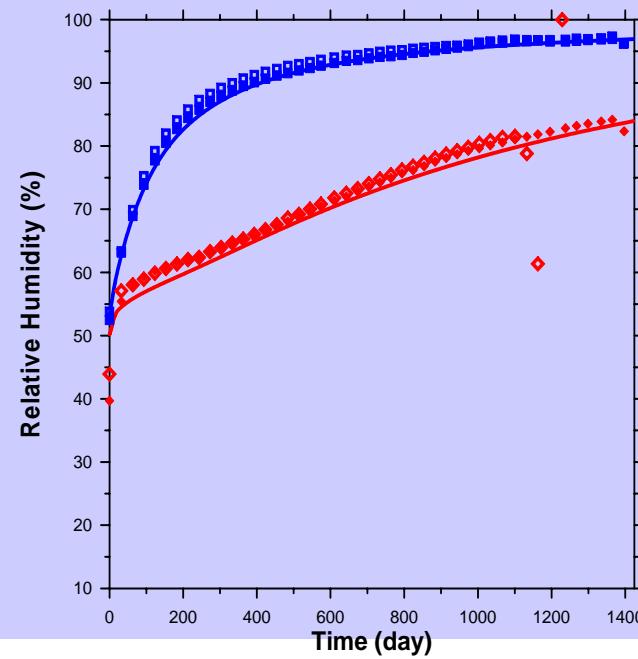
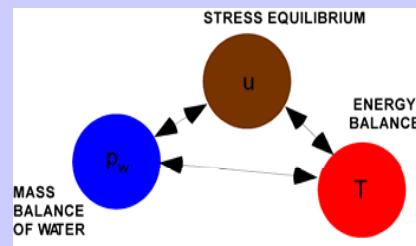
THM problem: Febex mock-up



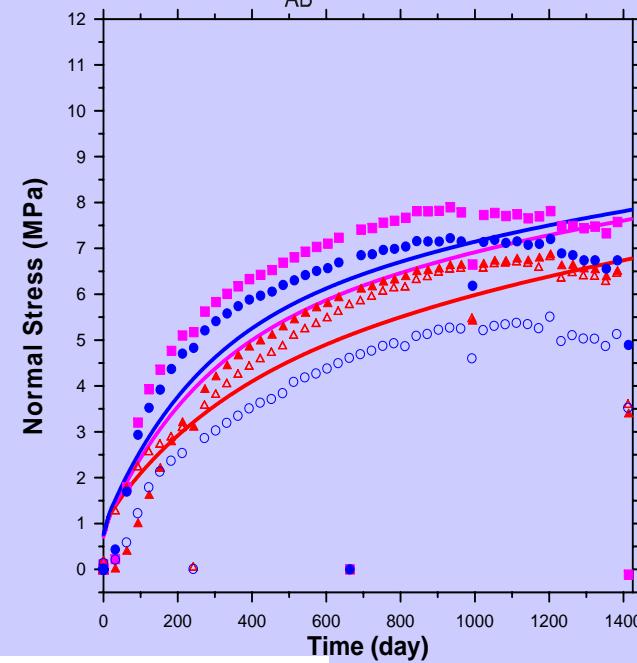
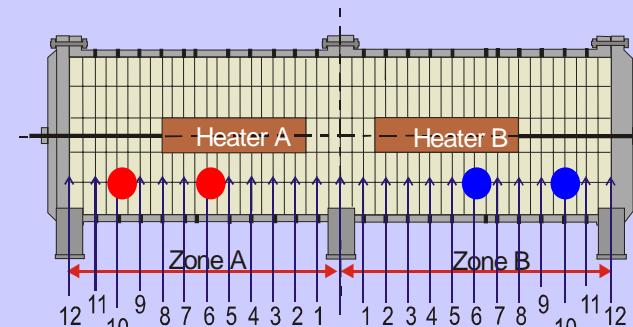
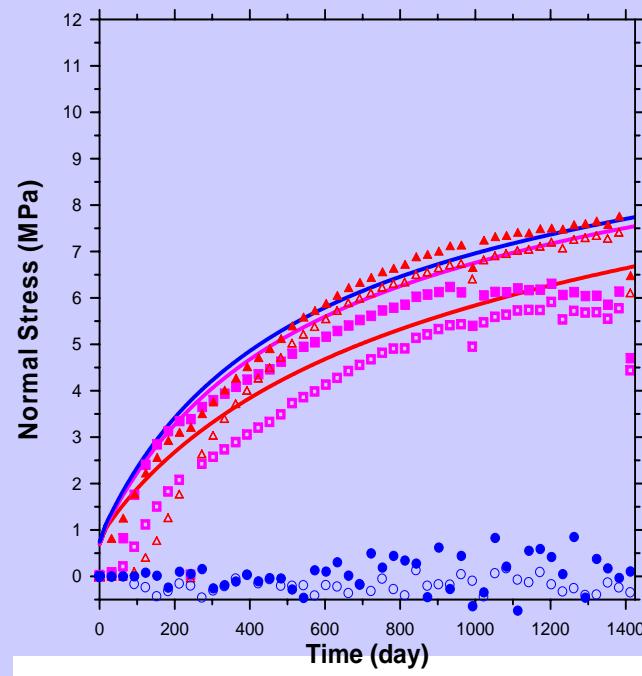
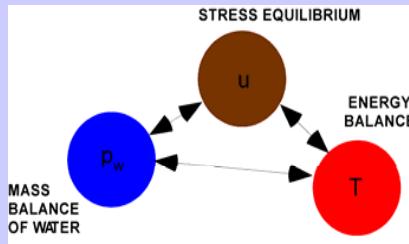
FEBEX MOCK-UP TEST SCHEMATIC DIAGRAM



THM problem: Febex mock-up



THM problem: Febex mock-up



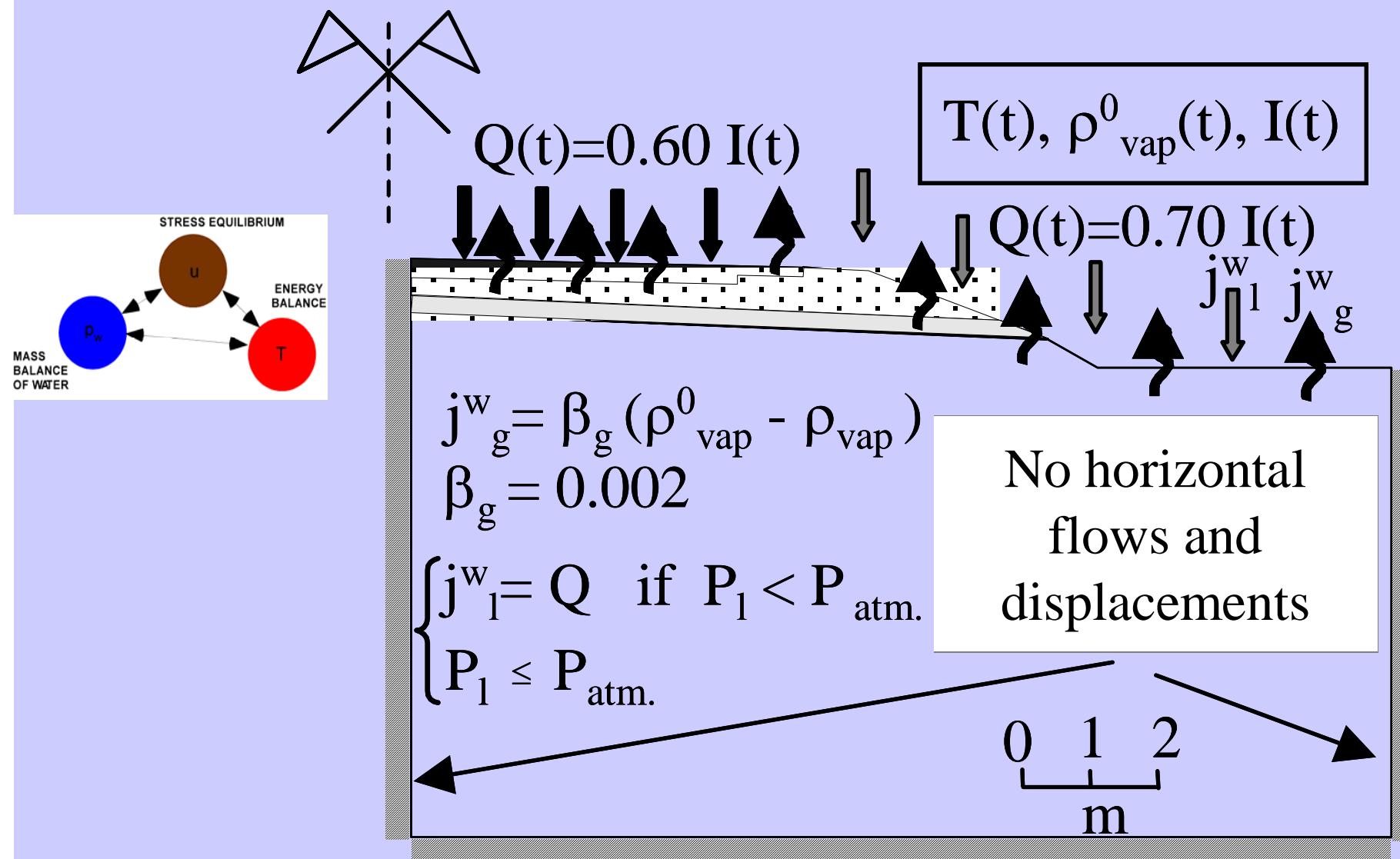
Boundary conditions

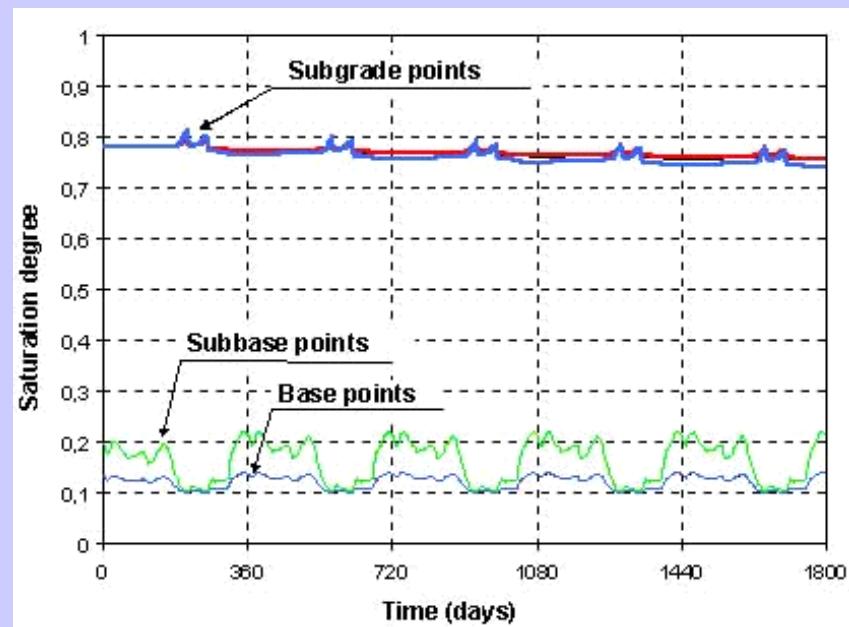
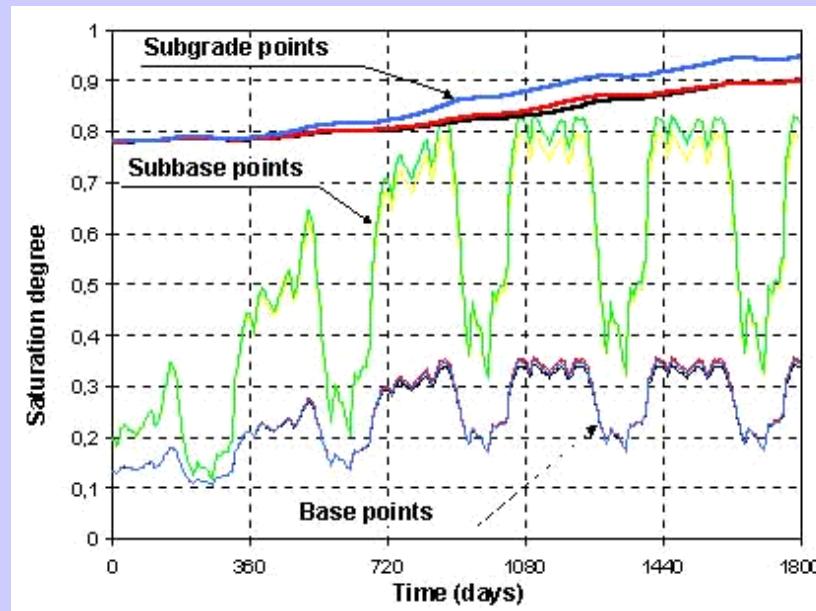
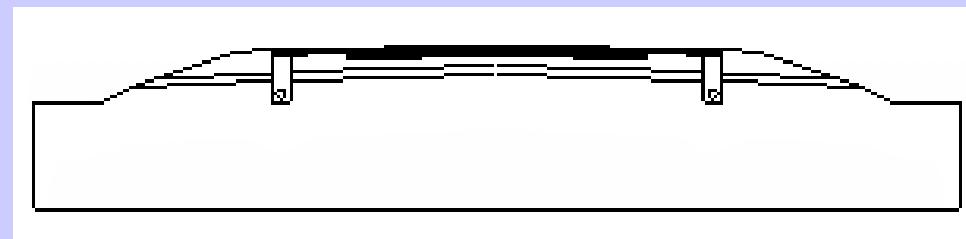
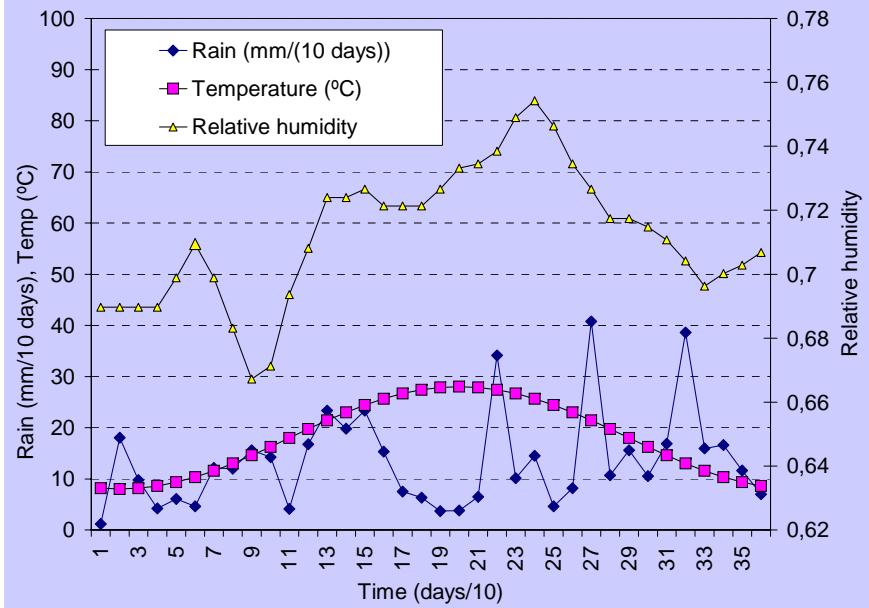
$$j_g^w = (\omega_g^w)^0 j_g^0 + (\omega_g^w)^0 \gamma_g (P_g^0 - P_g) + \beta_g \left((\rho_g \omega_g^w)^0 - (\rho_g \omega_g^w) \right)$$

where the superscript $(\cdot)^0$ stands for prescribed values.

- The **first term** is the mass inflow or outflow that takes place when a flow rate of gas (j_g^0) is prescribed.
- The **second term** is the mass inflow or outflow that takes place when gas phase pressure (P_g^0) is prescribed at a node. The coefficient γ_g is a leakage coefficient, i.e., a parameter that allows a boundary condition of the Cauchy type.
- The **third term** is the mass inflow or outflow that takes place when vapour mass fraction is prescribed at the boundary. This term naturally comes from the nonadvection flux (Fick's law). Mass fraction and density prescribed values are only required when inflow takes place. For outflow the values in the medium are considered.

ROAD PAVEMENT PROBLEMS





Formulación para
problemas THM

Constitutive laws

Darcy \Rightarrow liquid and gas flow

Fick \Rightarrow vapor and air diffusion (dispersion)

Fourier \Rightarrow heat conduction

Retention curve \Rightarrow saturation versus suction

Water V-P-T \Rightarrow water density

Ideal gases law \Rightarrow gas density

Henry's law \Rightarrow dissolved air

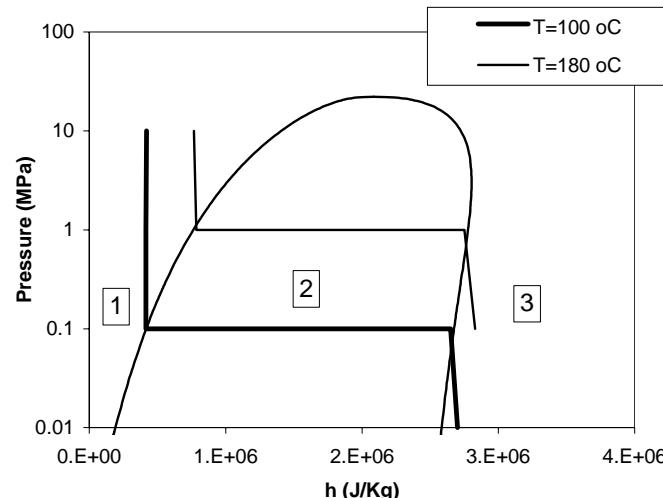
Psychrometric law \Rightarrow vapour concentration

Mechanical model \Rightarrow stress – strain response

Enthalpy of water

Depending on the pressure of water (p , MPa) and the enthalpy per unit mass (h , J/kg) three regions are distinguished.

From 100 °C to 180 °C, vapor pressure changes by one order of magnitude. While atmospheric pressure (0.1 MPa) is the vapor pressure at 100 °C, 1 MPa is the vapor pressure at 180 °C.



- (1) Single phase region
(liquid water)**
- (2) two phase region
(liquid water+vapor)**
- (3) single phase region
(heated vapor)**

Pressure-enthalpy diagram for pure water. Isotherm lines for 100 °C and 180 °C are represented.

SOIL DESATURATION

- Liquid pressure decrease or air pressure increase:
two phase flow with nearly immiscible fluids.

$P_g \approx P_a$ (*moderate T*).

- Vapour pressure increase: gas pressure is dominated by vapour pressure.

$P_g \approx P_v$ (*relatively high T*).

Thermal conductivity

Fourier's law to compute conductive heat flux, i.e.:

$$\mathbf{i}_c = -\lambda \nabla T$$

Where λ is thermal conductivity of the porous medium.

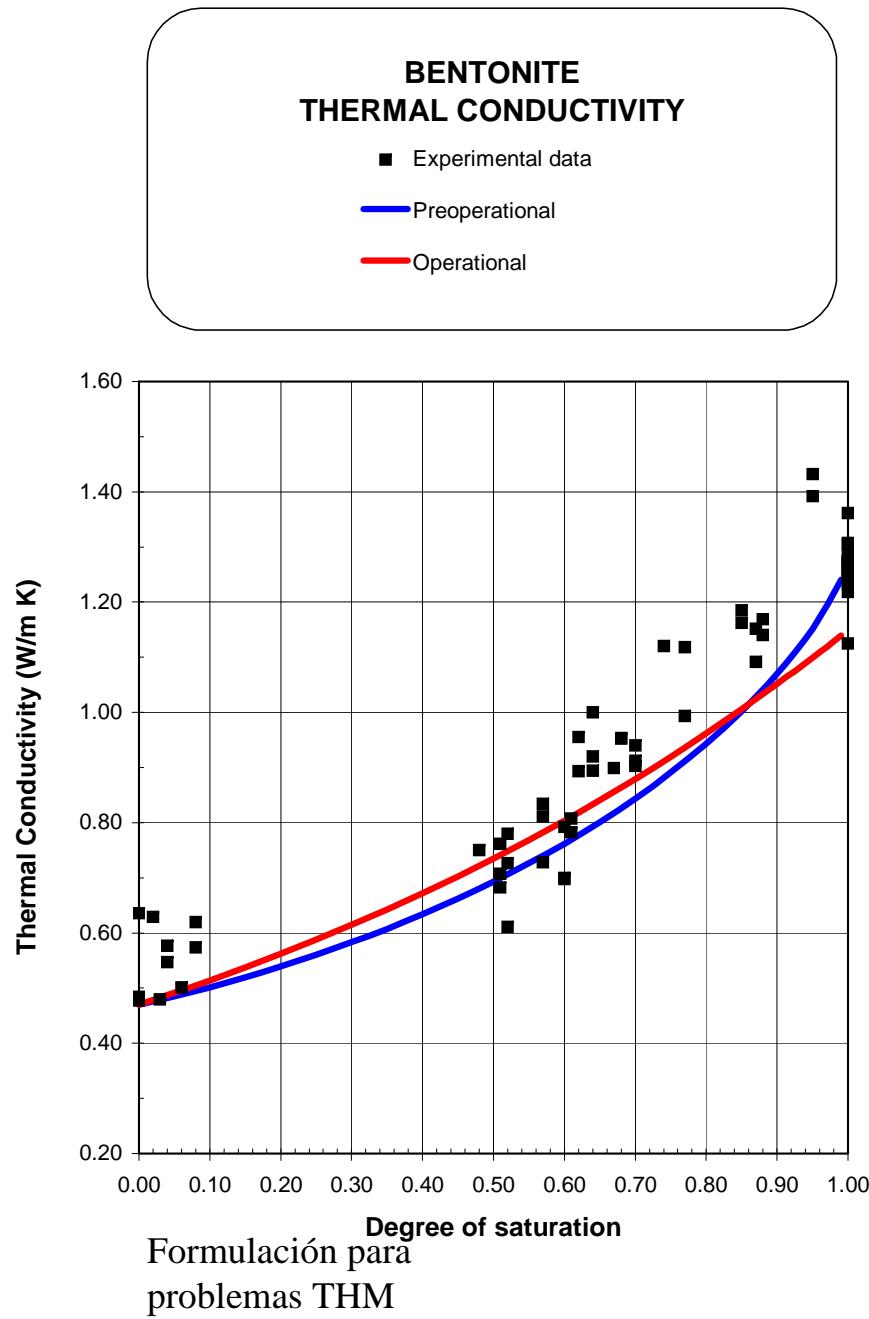
$$\lambda_{dry} = \lambda_{solid}^{(1-\phi)} \lambda_{gas}^{\phi} \quad \lambda_{sat} = \lambda_{solid}^{(1-\phi)} \lambda_{liq}^{\phi}$$

$$\lambda_{solid} = (\lambda_{solid})_o + a_1 \mathbf{T} + a_2 \mathbf{T}^2 + a_3 \mathbf{T}^3$$

The definition of *dry* and *saturated* thermal conductivities permits to separate the dependence on porosity and the dependence on degree of saturation. Finally,

$$\lambda = \lambda_{sat}^{S_l} \lambda_{dry}^{(1-S_l)}$$

On the other hand, the *dry* and *saturated* thermal conductivities can be calculated as described above or directly determined experimentally.



Thermal conductivity

Thermal conductivity is used in Fourier's law to compute conductive heat flux, i.e.:

$$\mathbf{i}_c = -\lambda \nabla T$$

with

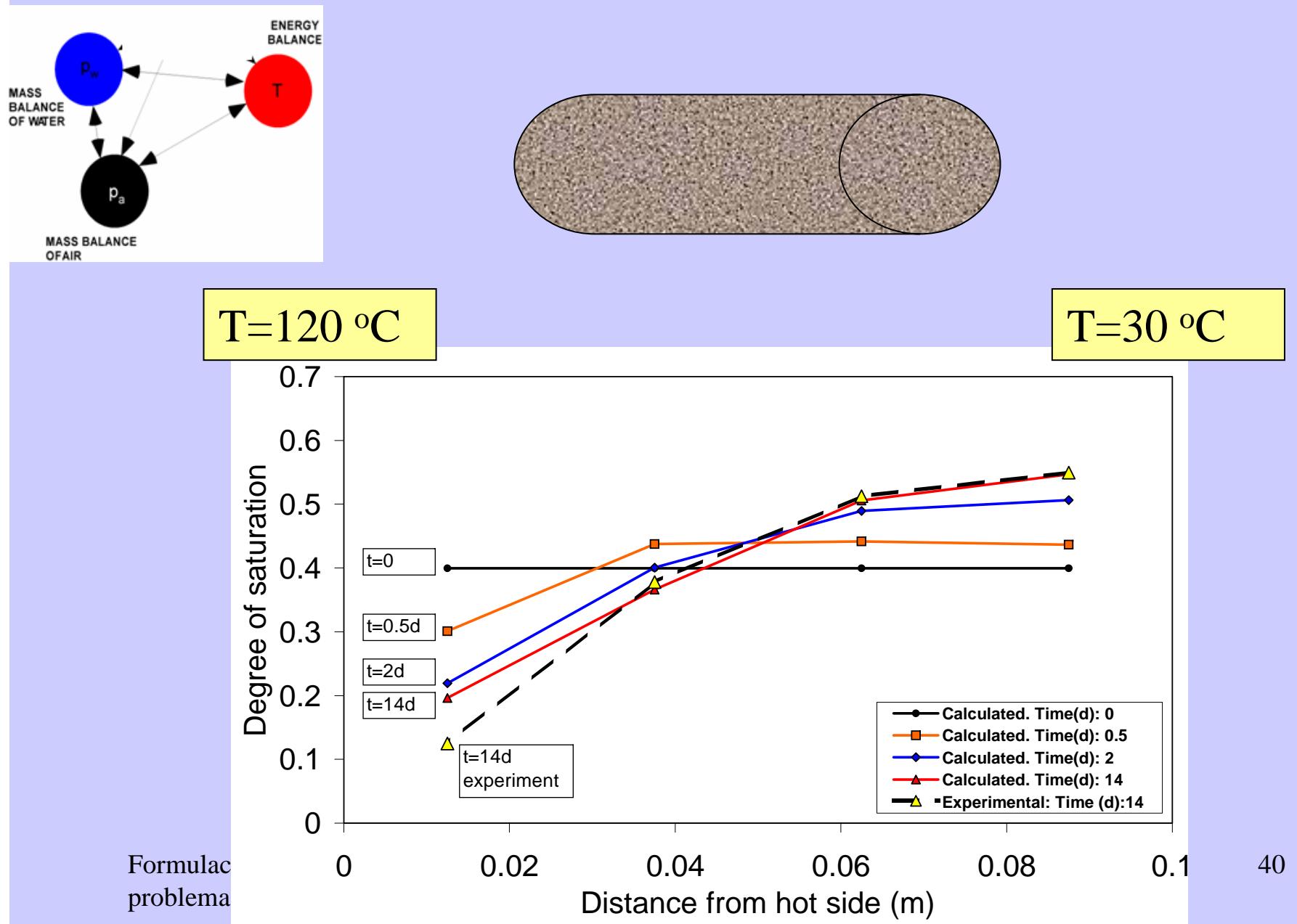
$$\lambda = \lambda_{sat}^{S_l} \lambda_{dry}^{(1-S_l)}$$

$$\lambda_{dry} = \lambda_{solid}^{(1-\phi)} \lambda_{gas}^{\phi}$$

$$\lambda_{sat} = \lambda_{solid}^{(1-\phi)} \lambda_{liq}^{\phi}$$

$$\lambda_{solid} = (\lambda_{solid})_o + a_1 T + a_2 T^2 + a_3 T^3$$

Numerical modelling of drying induced by temperature gradients



Liquid phase density

As a first approximation, the density of liquid phase can be calculated as:

$$\rho_l = \rho_{l0} \exp(\beta(P_l - P_{l0}) + \alpha T + \gamma c)$$

where a water compressibility (β) and a volumetric expansion coefficient (α) are defined. Reference values of $4.5 \cdot 10^{-4} \text{ MPa}^{-1}$ and $-3.4 \cdot 10^{-4} \text{ }^{\circ}\text{C}^{-1}$ can be used in this expression.

Gas phase density

Gas density is calculated as the sum of vapor and air density. This can be written as:

$$\rho_g = \rho_v + \rho_a$$

If ideal gases law is used to calculate vapor and air density, it follows that:

$$\rho_g = \rho_v + \rho_a = \frac{P_v M_v}{R T} + \frac{P_a M_a}{R T}$$

where the $M_v = 0.018 \text{ kg/mol}$ and $M_a = 0.028 \text{ kg/mol}$ are the molecular masses for vapor and dry-air. R is the ideal gas constant (8.3143 J/molK) and T is temperature. On the other hand, partial pressures principle is assumed to hold in the vapor-air mixture ($P_g = P_v + P_a$), which implies that:

$$M_g = \frac{P_v M_v + P_a M_a}{P_g}$$

i.e. the gas phase has variable molecular mass depending on its composition.

If P_g is given, air pressure can be calculated as $P_a = P_g - P_v$ where it is assumed that vapor pressure is also known.

Vapor partial pressure

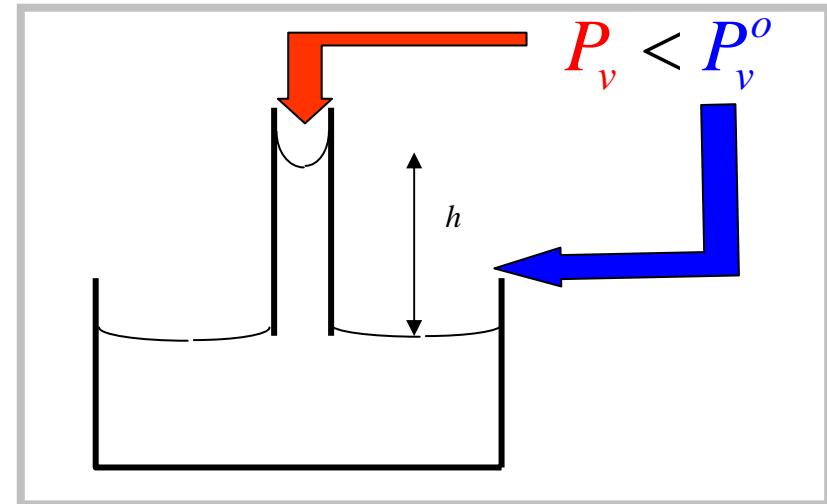
Vapor pressure depends on temperature and suction:

$$P_v(T, s) = P_v(T) \times F(s, T)$$

$$P_v(T) = 136075 \exp\left(\frac{-5239.7}{273 + T}\right)$$

$$F(P_c, T) = \exp\left(\frac{-s M_w}{R(273 + T) \rho_l}\right)$$

$$s = P_g - P_l$$



In which $P_v(T)$ is the boundary between (1) and (2) in the phase diagram for pure water and $F(s, T)$ is a reduction term due to capillary effects (psychrometric law).

Relative humidity inside soil pores is assumed to be F

Retention curve

This law relates capillary pressure and degree of saturation. One of the commonly used models is the van Genuchten model (van Genuchten, 1980) that can be written as:

$$S_e = \frac{S_l - S_{\min}}{S_{\max} - S_{\min}} = \left(1 + \left(\frac{P_g - P_l}{P} \right)^{1/(1-\lambda)} \right)^{-\lambda}$$

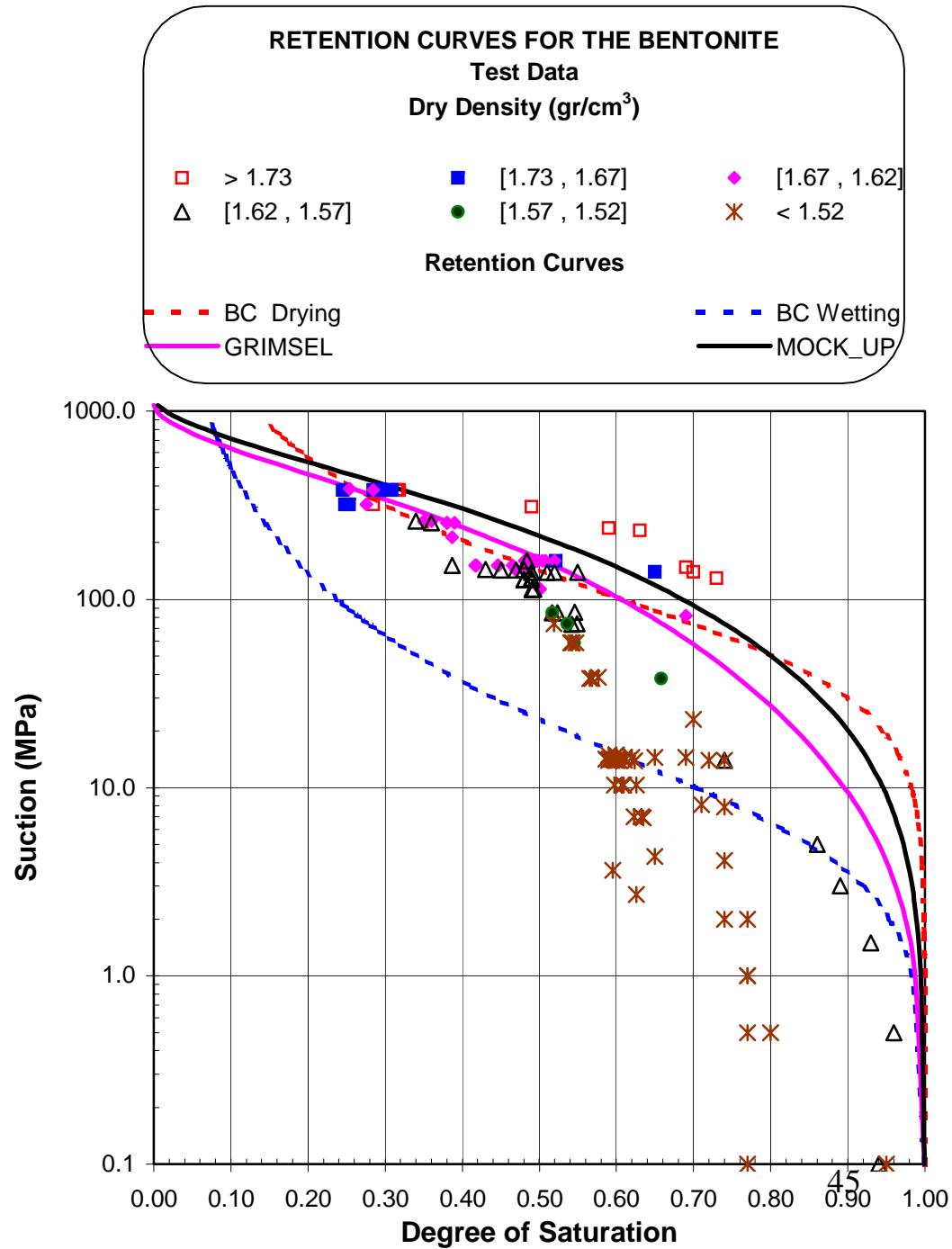
This equation contains the parameters λ and P . The first (λ) essentially controls the shape of the curve while the second (P) controls its height, so this latter can be interpreted as a measure of the capillary pressure required to start the desaturation of the soil. Since capillary pressure can be scaled with surface tension (Milly, 1982) it appears that P also does. This can be shown if Laplace's law is recalled:

$$P_g - P_l = \frac{2\sigma}{r} \quad P_g - P_l = P \left(S_e^{-1/\lambda} - 1 \right)^{(1-\lambda)} \quad P = P_o \frac{\sigma(T)}{\sigma(T_o)}$$

where σ is surface tension and r is the curvature radius of the meniscus.

Experimental retention curve

Van Genuchten law
Different densities
Swelling not permitted (except in low density samples)



Darcy's law

Advective fluxes of liquid and gas are calculated using darcy's law in the generalized form:

$$\mathbf{q}_\alpha = -\frac{\mathbf{k} k_{r\alpha}}{\mu_\alpha} (\nabla P_\alpha - \rho_\alpha \mathbf{g})$$

Where $\alpha = l, g$ if the law is used for liquid or gas phase. Intrinsic permeability (\mathbf{k}) relative permeability ($k_{r\alpha}$) and viscosity (μ_α) should be calculated.

Intrinsic permeability

This parameter depends primarily on the porous medium structure.

$$\mathbf{k} = \left[\mathbf{k}_o \frac{(1-\phi_o)^2}{\phi_o^3} \right] \frac{\phi^3}{(1-\phi)^2} = \mathbf{a} \frac{\phi^3}{(1-\phi)^2}$$

where ϕ_o is a reference porosity and \mathbf{k}_o is the intrinsic permeability at the reference porosity.

Intrinsic permeability

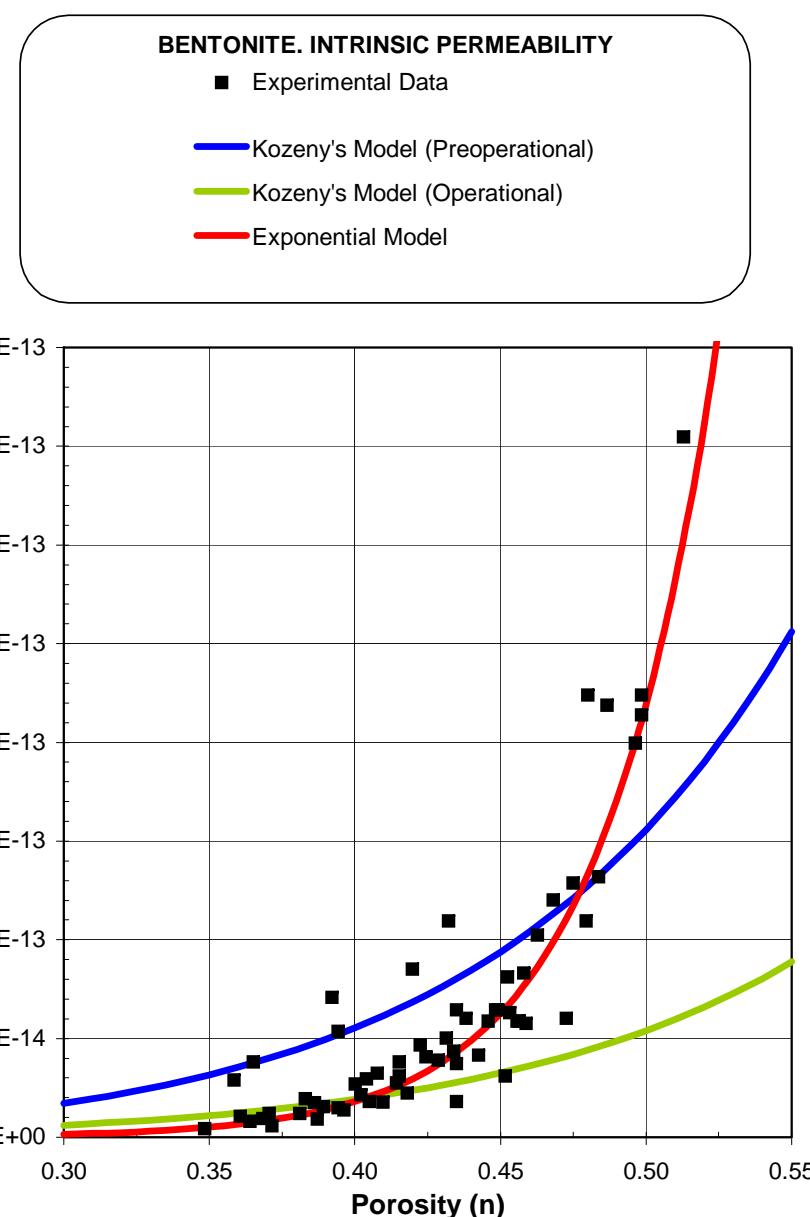
For a continuum medium (Kozeny's model)

$$\mathbf{k} = \mathbf{k}_o \frac{\phi^3}{(1-\phi)^2} \frac{(1-\phi_o)^2}{\phi_o^3}$$

ϕ_o : reference porosity

\mathbf{k}_o : intrinsic permeability for matrix ϕ_o
which is used in Darcy's law:

$$\mathbf{q}_\alpha = -\frac{\mathbf{k} k_{r\alpha}}{\mu_\alpha} (\nabla P_\alpha - \rho_\alpha \mathbf{g})$$



Liquid viscosity

Water viscosity is required in Darcy's law.

$$\mu_l = A \exp\left(\frac{B}{273.15 + T}\right)$$

Where $A = 2.1 \cdot 10^{-2}$ MPa s and $B = 1808.5$ K $^{-1}$.

Gas viscosity

$$\mu_g = \frac{A\sqrt{273+T}}{\left(1+\frac{B}{273+T}\right)} \frac{1}{1+\frac{b_k}{P_g}} \quad b_k = C - Dk \geq 0 \quad (k : \text{intrinsic permeability})$$

Where $A = 1.48 \cdot 10^{-12}$ MPa s, $B = 119.4$ K $^{-1}$, $C = 0.14$ and $D = 1.2 \cdot 10^{15}$. A reduction coefficient takes into account the possibility of Knudsen diffusion (Klinkenberg effect). As the mean free path of the gas particles becomes comparable to the pore size (low permeability material) slip effects (non laminar flow) may result in an apparent lower viscosity (apparent higher gas conductivity).

$$\mathbf{q}_\alpha = -\frac{\mathbf{k}k_{ra}}{\mu_g^o} \left(1 + \frac{b_k}{P_g}\right) (\nabla P_\alpha - \rho_\alpha \mathbf{g}) = -\frac{\mathbf{k}k_{ra}}{\mu_g^o} (\nabla P_\alpha - \rho_\alpha \mathbf{g}) - \left(\frac{\mathbf{k}k_{ra}}{\mu_g^o} \frac{b_k}{P_g}\right) (\nabla P_\alpha - \rho_\alpha \mathbf{g})$$

Fick's Law. Molecular Diffusion

To calculate vapor and dissolved air migration:

$$\mathbf{i}_\alpha^i = -(\phi \rho_\alpha S_\alpha \tau D_m^i \mathbf{I}) \nabla \omega_\alpha^i$$

where $\alpha=l, g$ and $i=a, w$ depending if diffusion takes place in the liquid (dissolved air) or in the gas phase (vapor).

$$D_m^{vap} = D \left(\frac{(273 + \textcolor{red}{T})^n}{P_g} \right)$$

where τ is a tortuosity reduction coefficient, $D = 5.9 \cdot 10^{-6} \text{ m}^2/\text{s}/\text{K}^{-n}\text{Pa}$ and $n=2.3$.

In the case of dissolved air, the following expression is used for molecular diffusivity:

$$D_m = D \exp \left(\frac{-Q}{R(273 + \textcolor{red}{T})} \right)$$

Where τ is a tortuosity reduction coefficient, $D = 1.1 \times 10^{-4} \text{ m}^2/\text{s}$ and $Q = 24530 \text{ J/mol}$.

Mechanical constitutive model

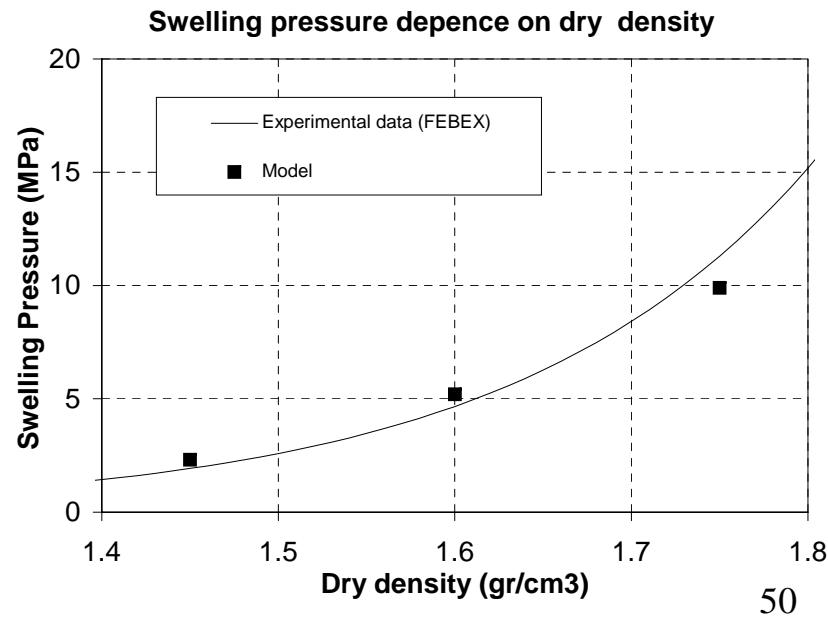
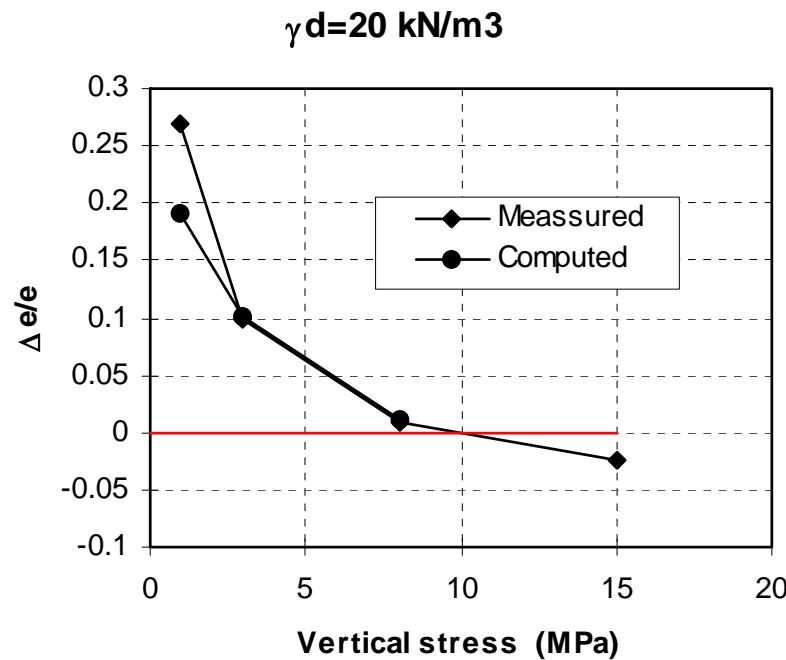
Stress-strain relationship: irreversible

Suction induces swelling and/or collapse

Strength depends on suction

Effective stress, suction, temperature

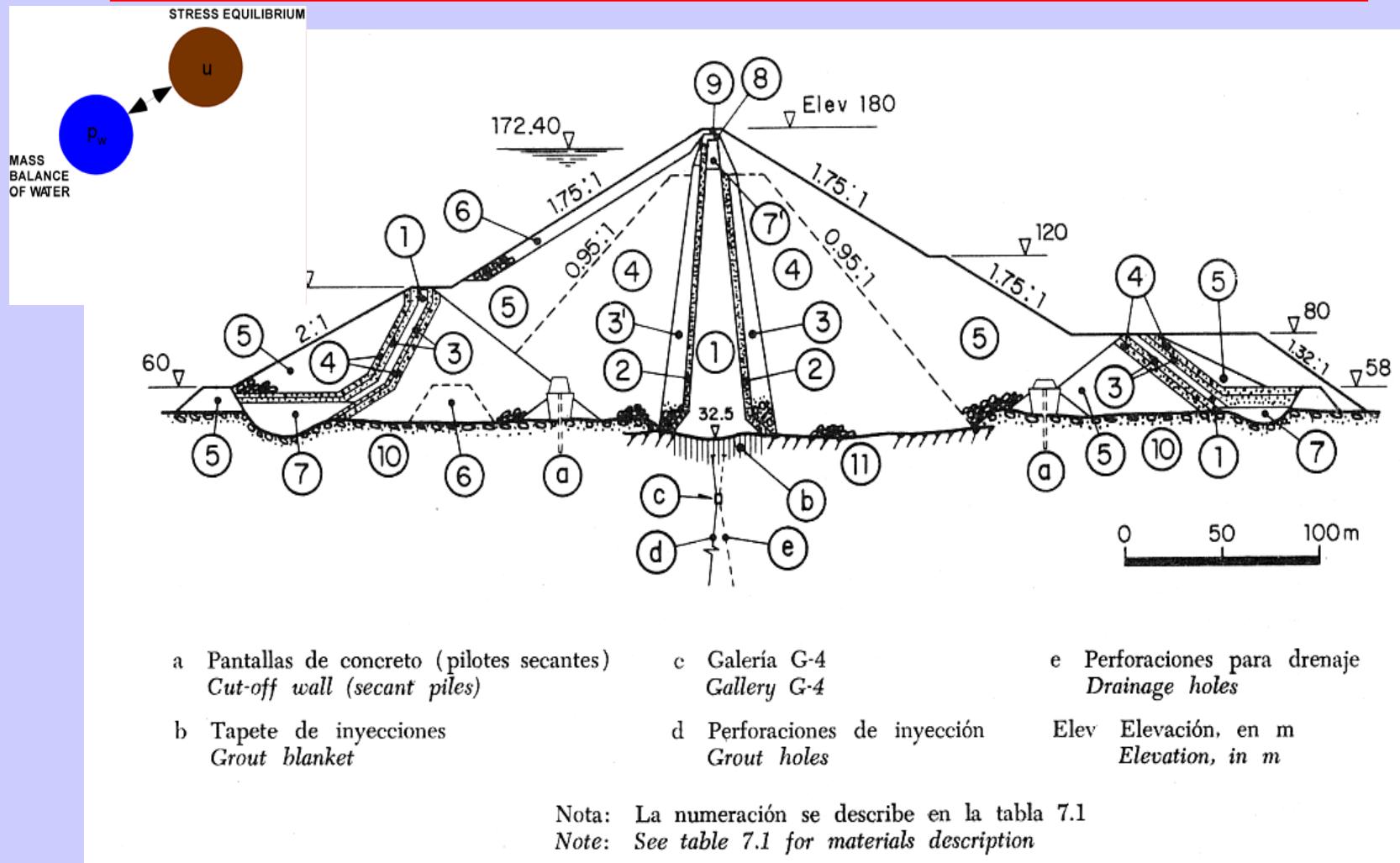
Nonlinear laws



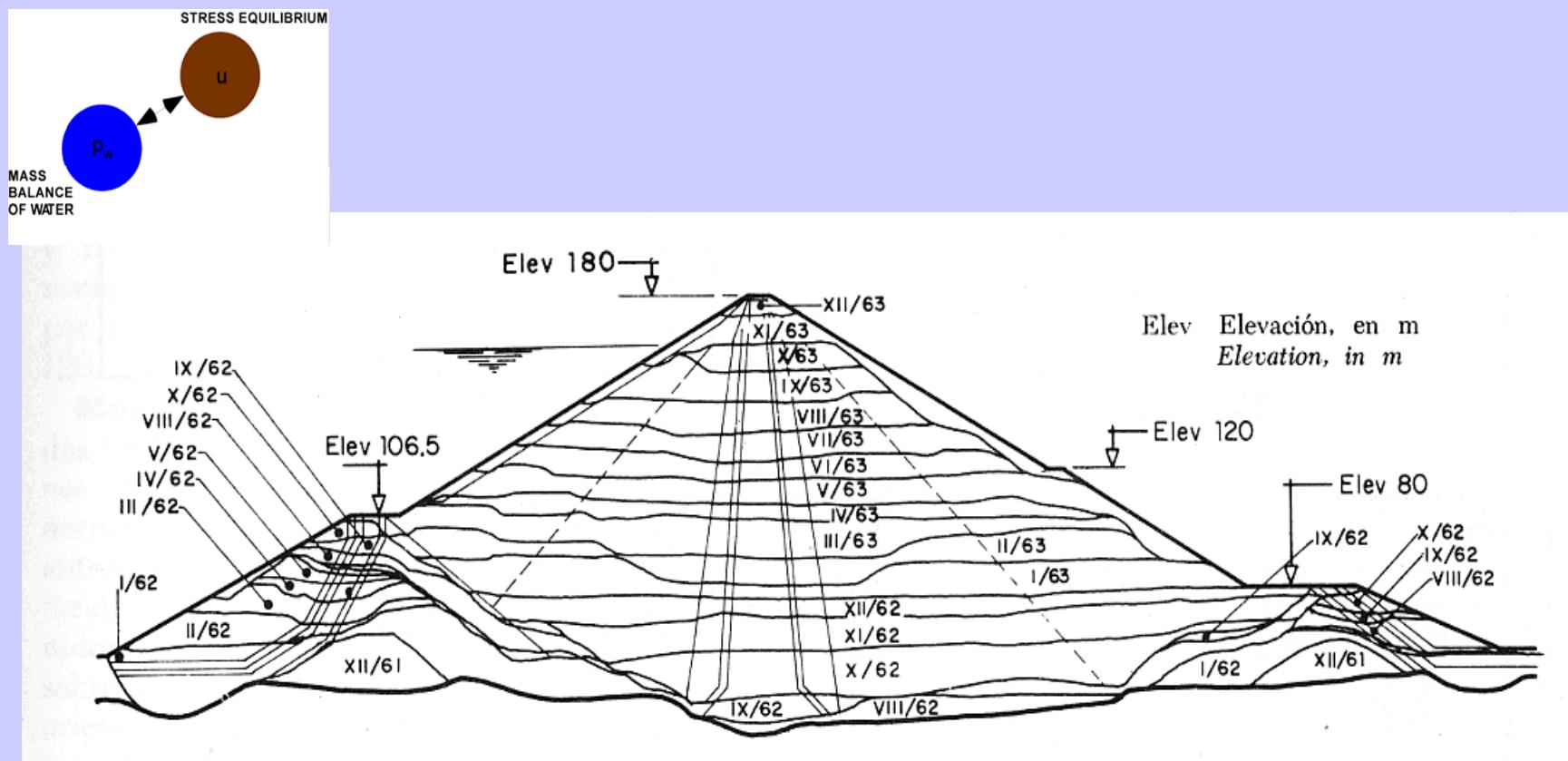
3. Algunos ejemplos

- Earth dams (2 types)
- Two phase flow
- Rock deformation at great depth
- Radioactive Waste disposal

HM problem: earth dams

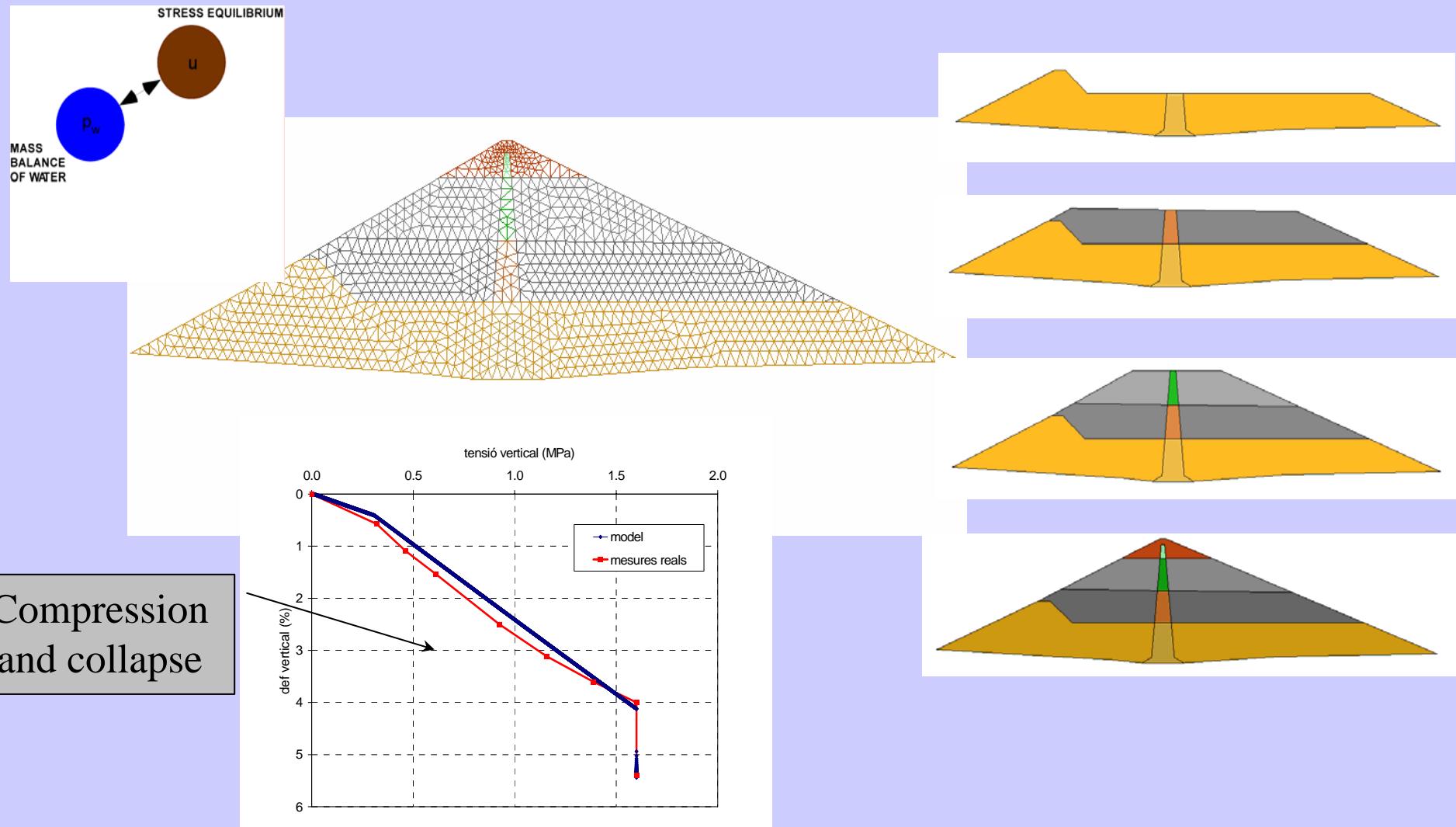


HM problem: earth dams



Construction history

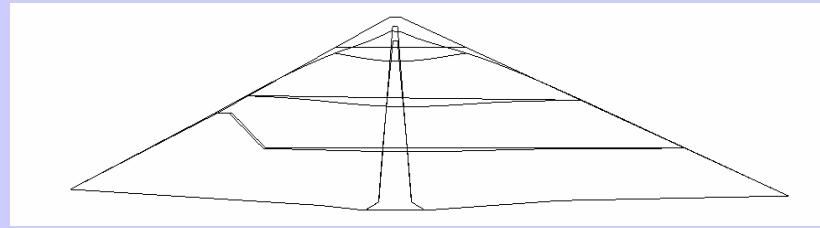
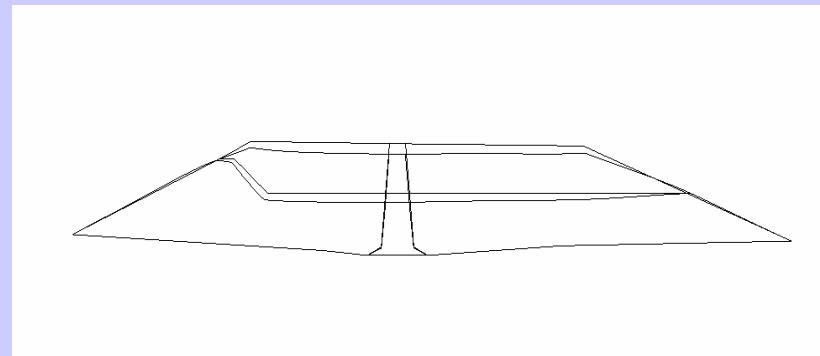
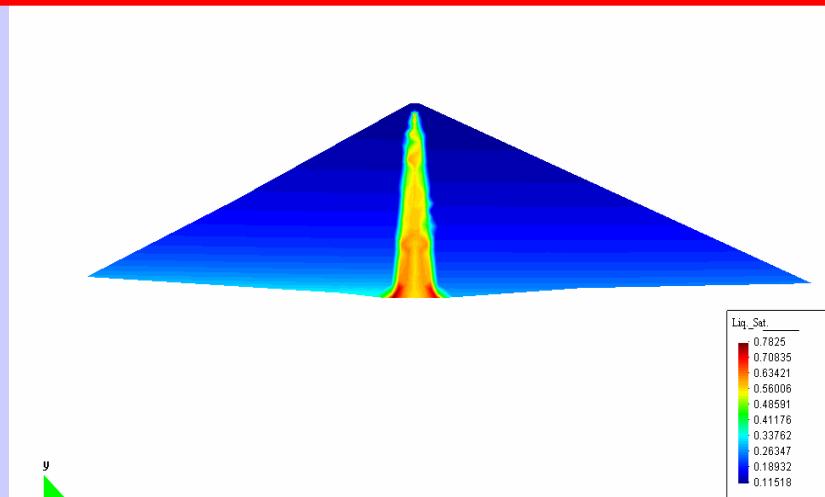
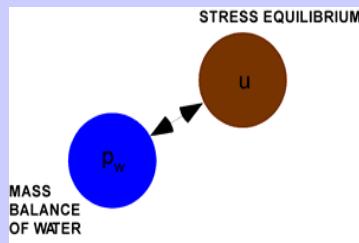
HM problem: earth dams



Compression
and collapse

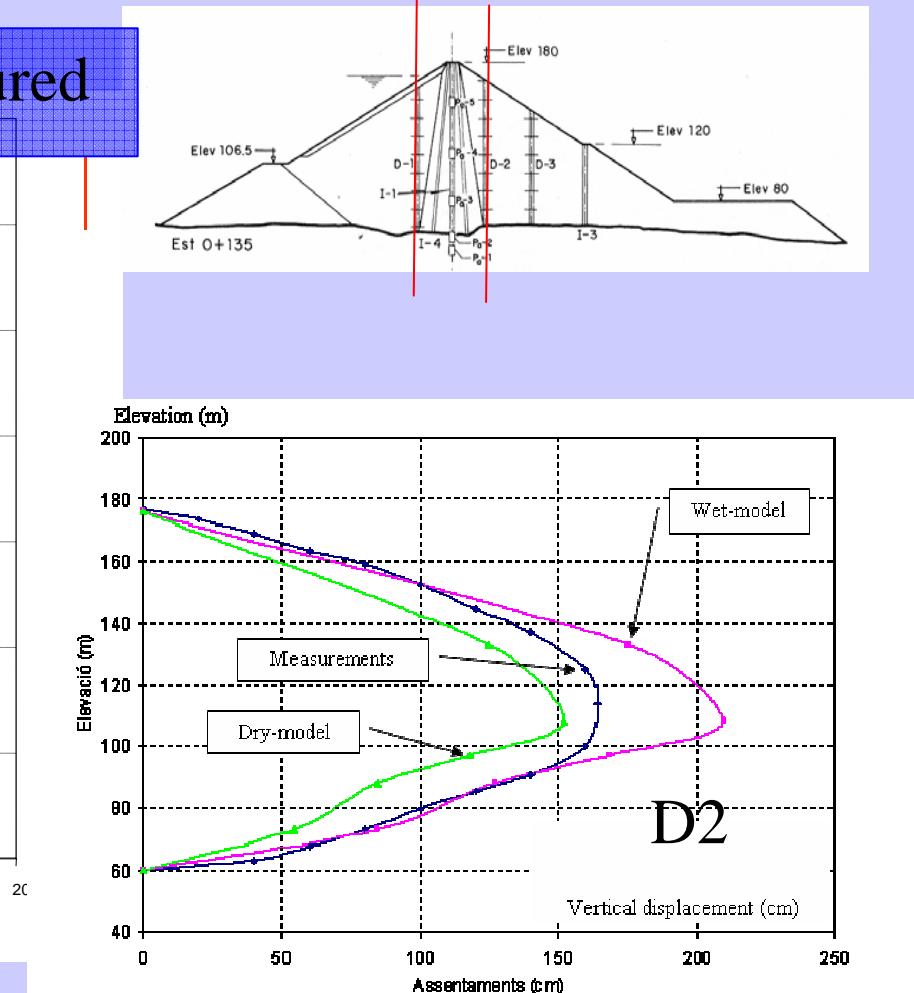
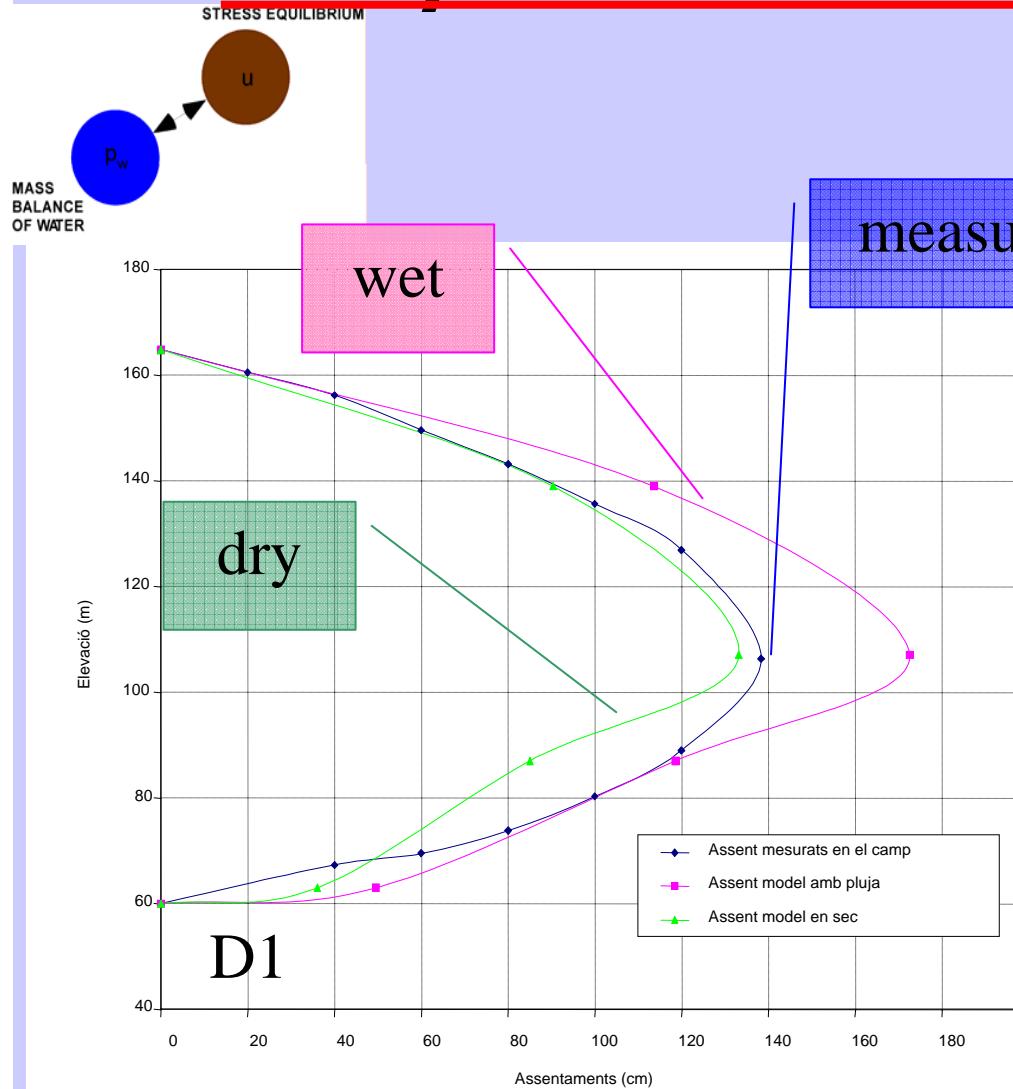
Formulación para
problemas THM

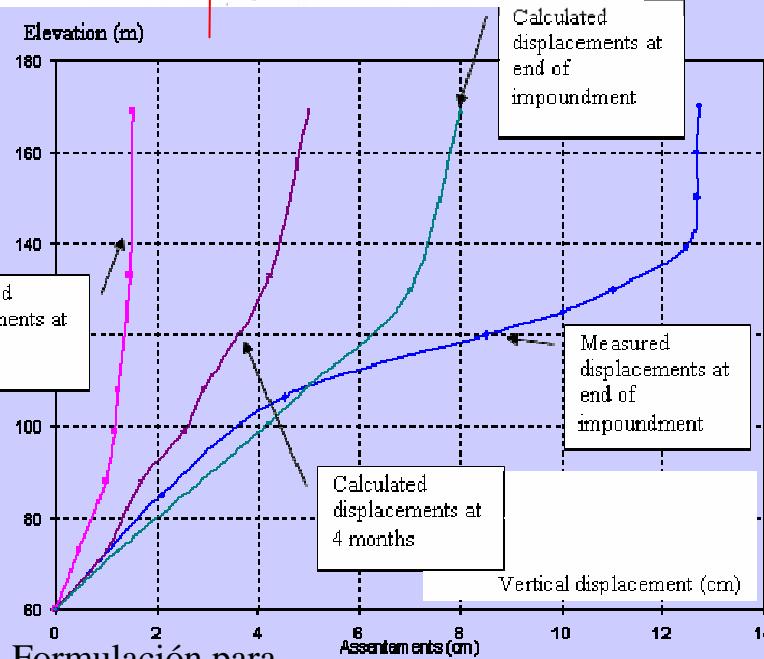
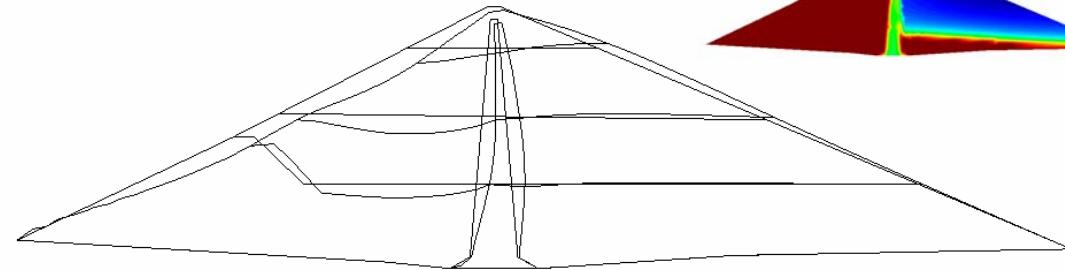
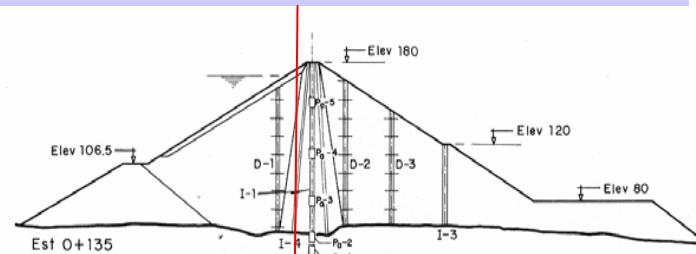
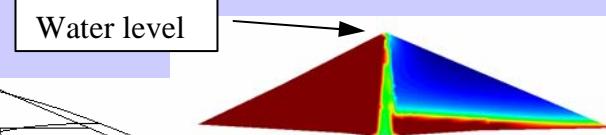
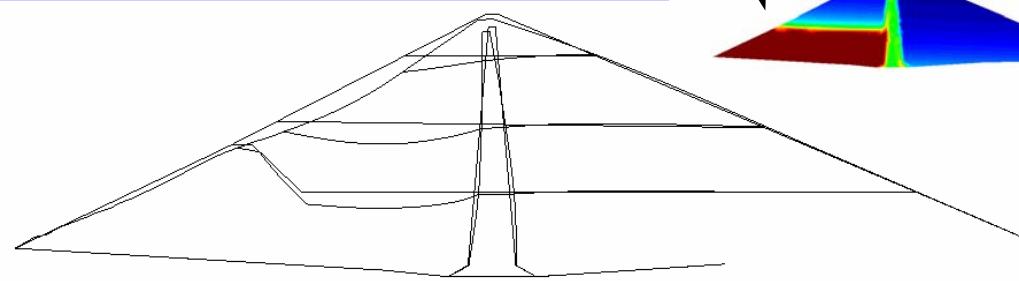
HM problem: earth dams



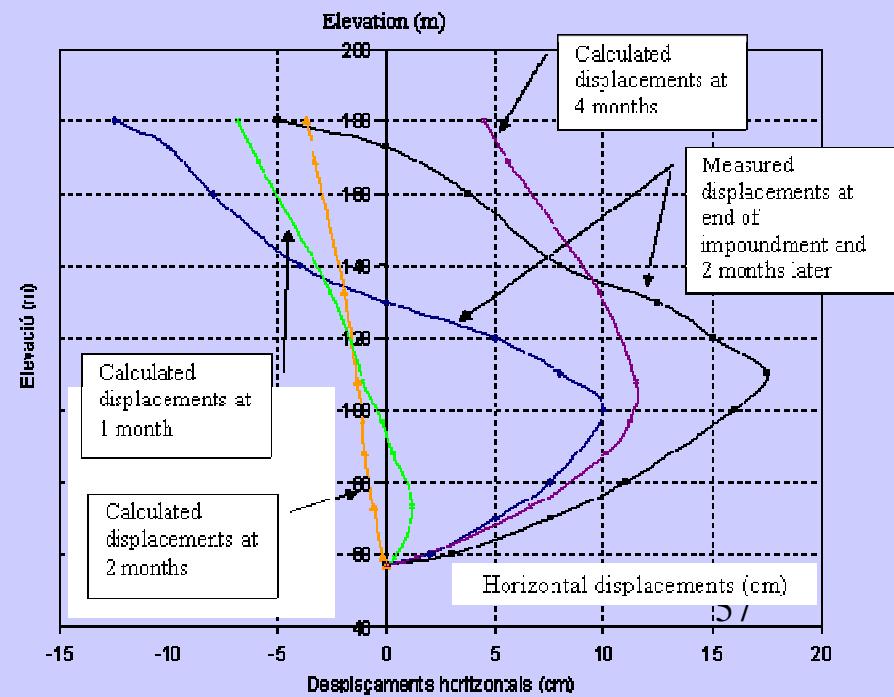
Formulación para
problemas THM

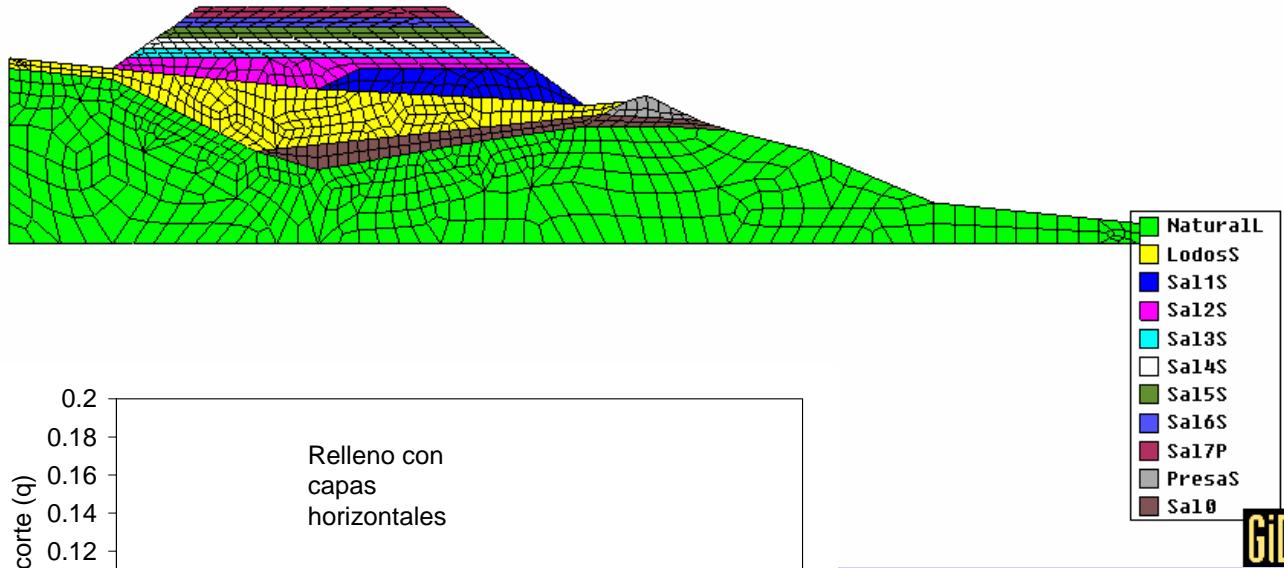
HM problem: dam construction



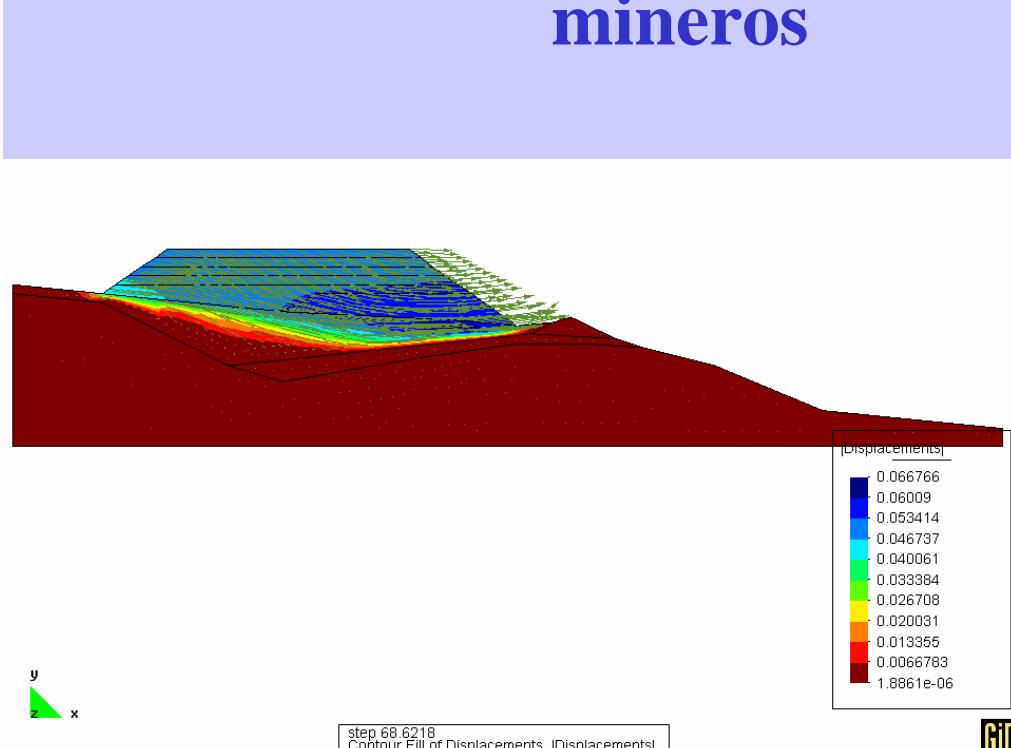
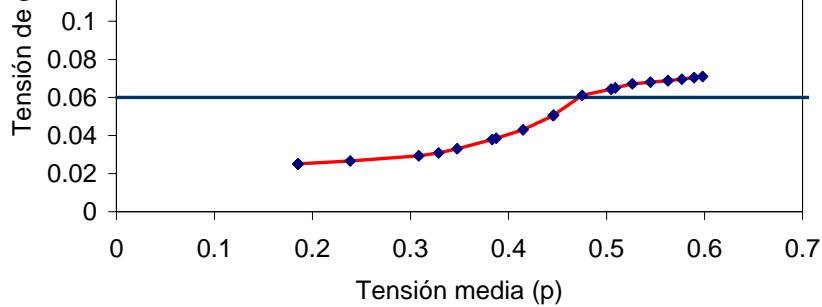


Formulación para
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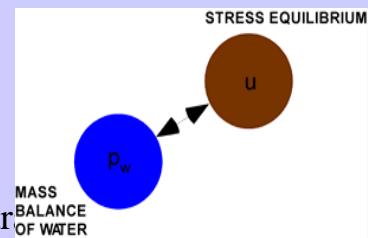




M y HM: Escombrera de residuos salinos sobre lodos mineros

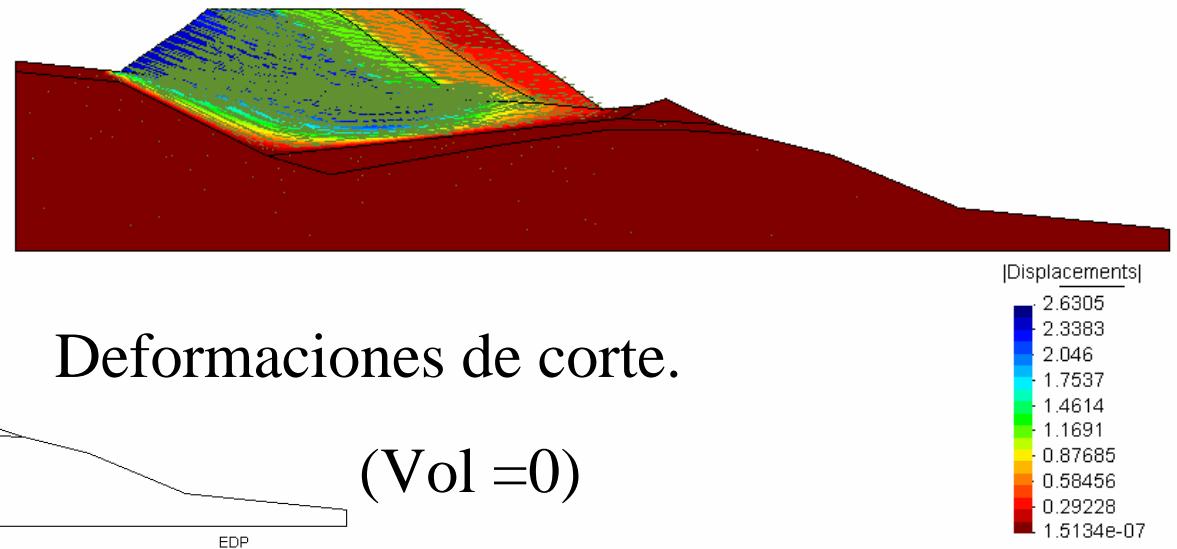


$$Cu = 30 \text{ kPa}$$

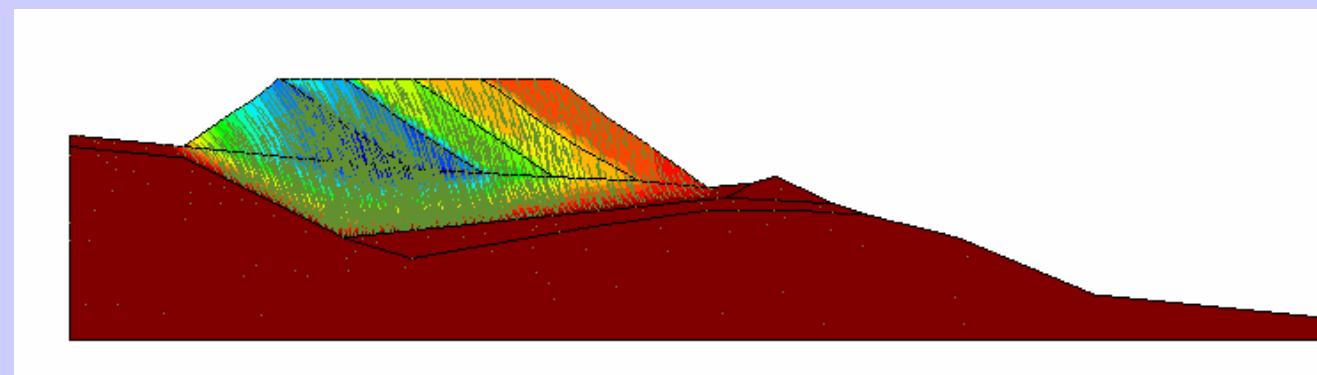


Formulación para
problemas THM

No drenado

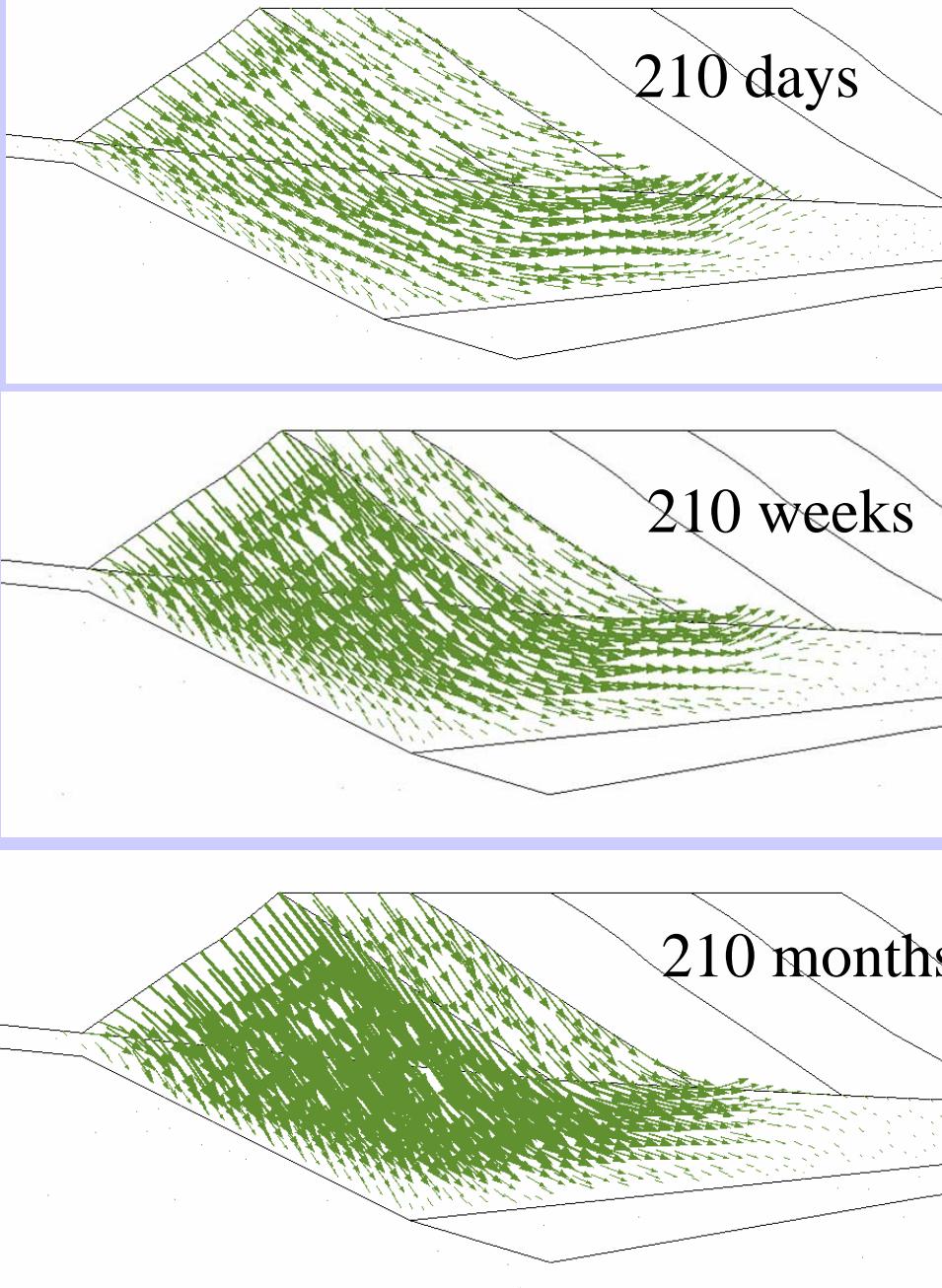


Drenado



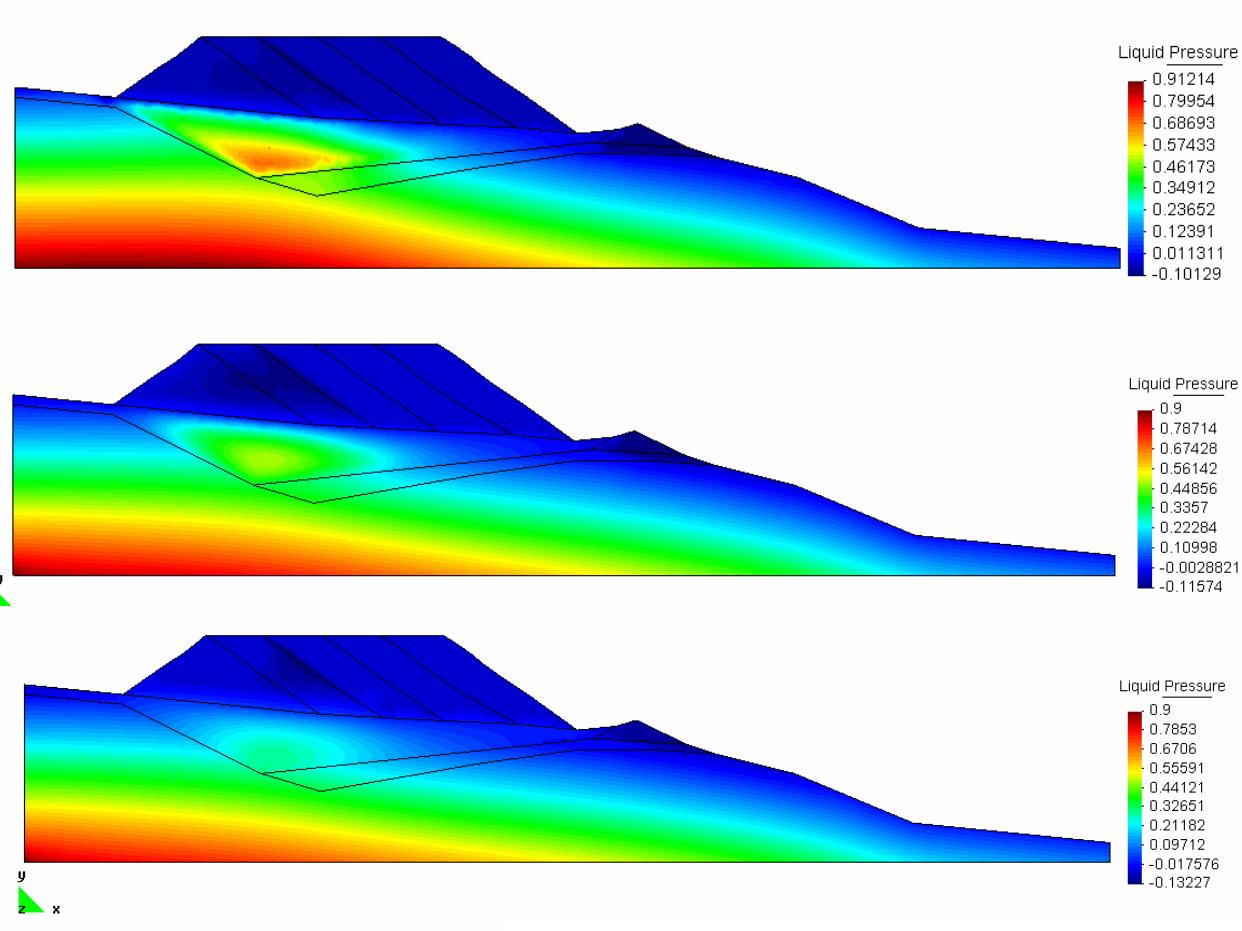
Formulación para
problemas THM

Deformaciones volumétricas
(hardening)



Formulación para
problemas THM

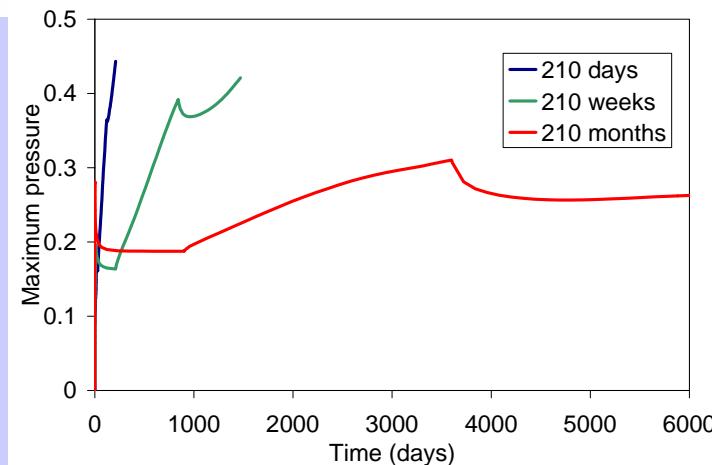
Desplazamientos para los casos analizados con 210 días (rápido), 210 semanas (lento) y 210 meses (muy lento) al final de la colocación de la 2^a etapa.



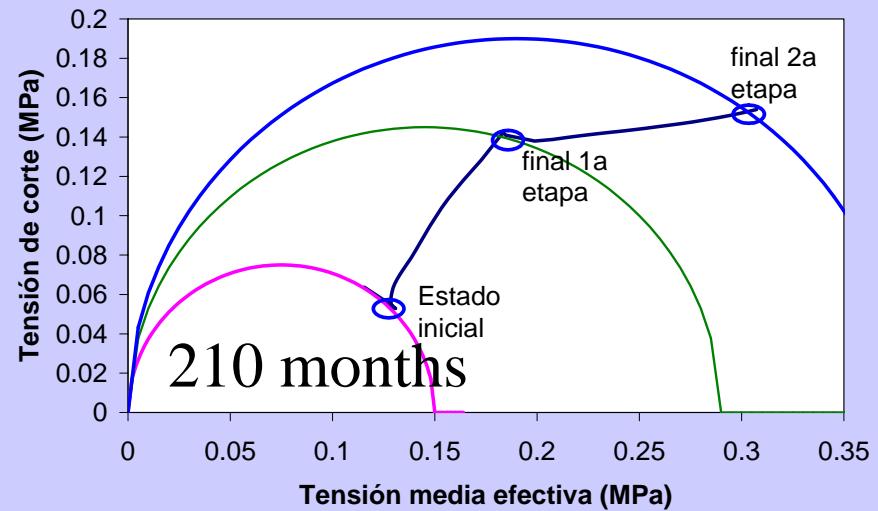
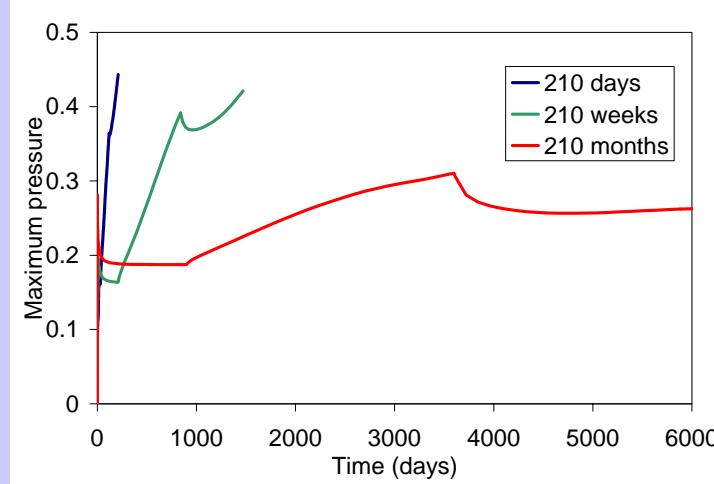
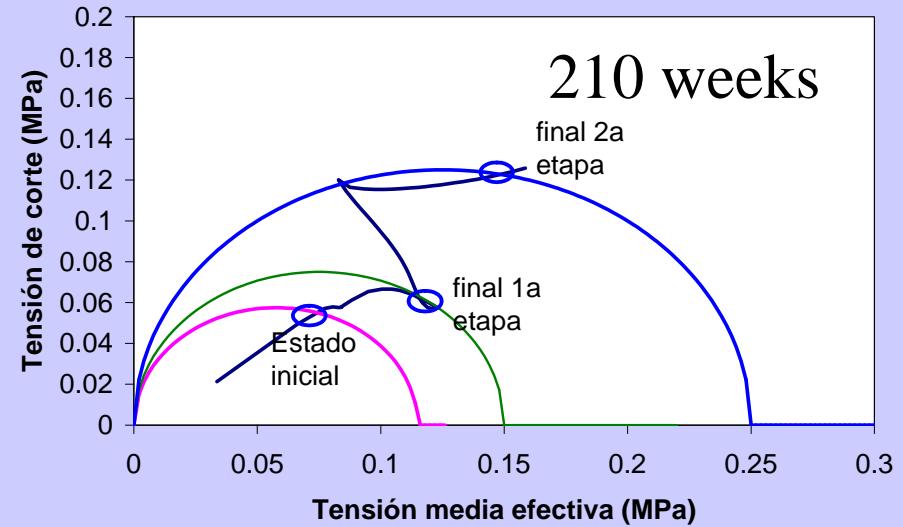
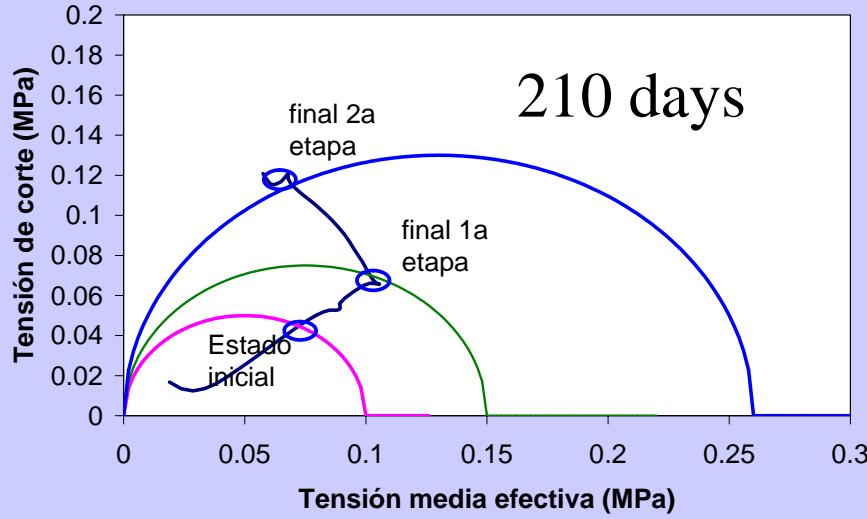
210 days

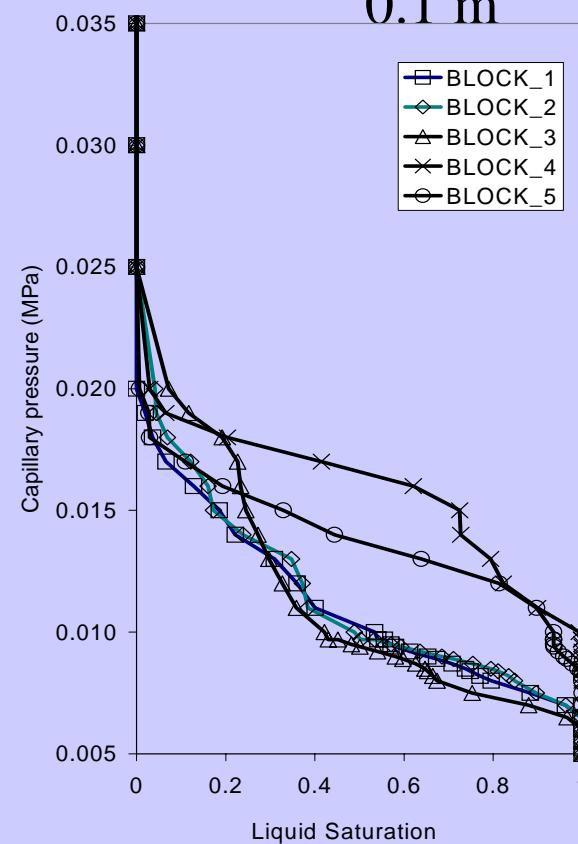
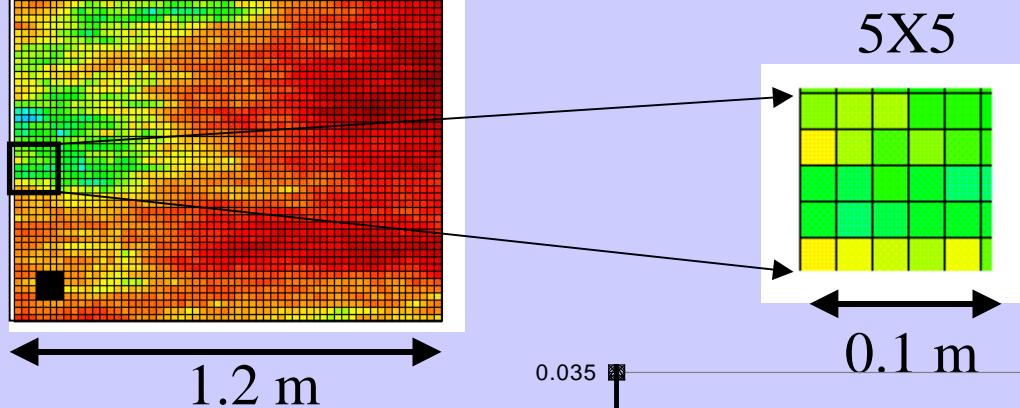
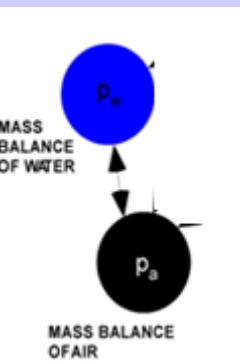
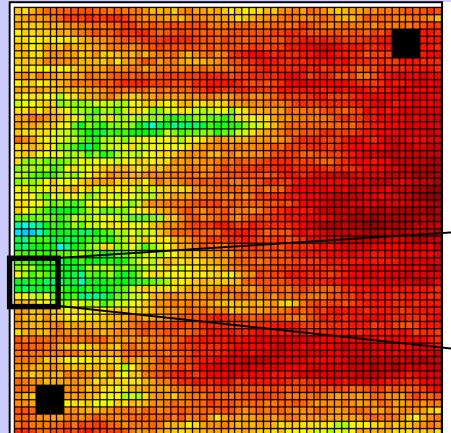
210 weeks

210 months

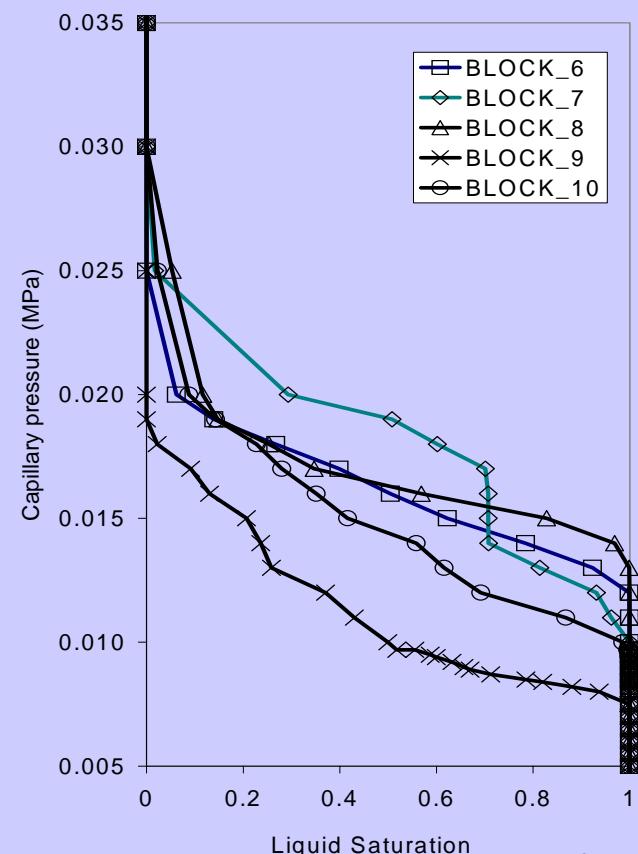


Formulación para
problemas THM

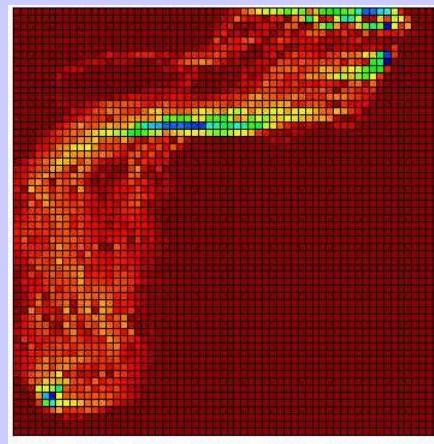
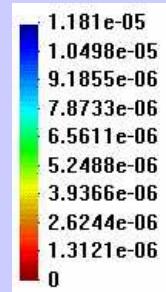




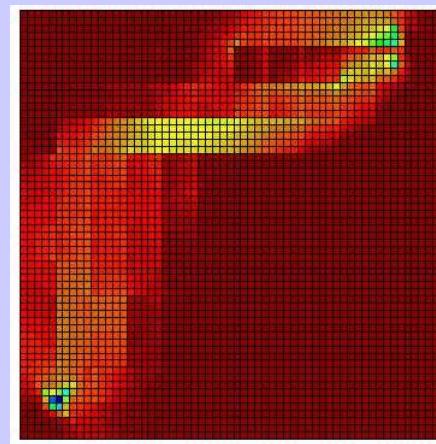
H: GAS AND WATER FLOW IN A PLANAR FRACTURE. THE UPSCALING PROBLEM



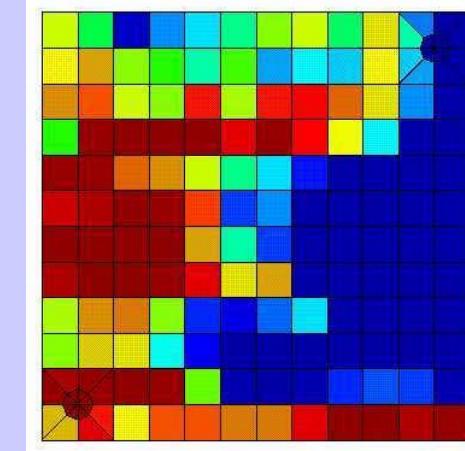
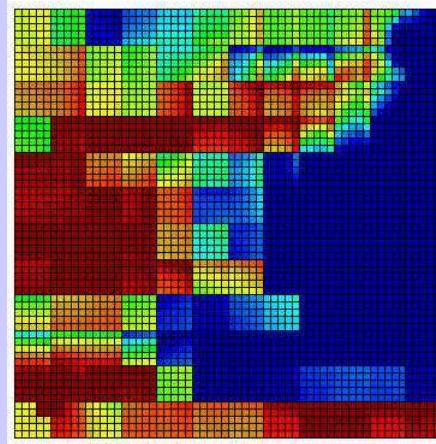
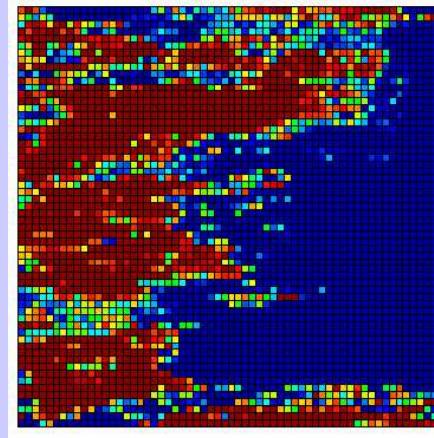
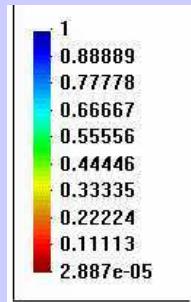
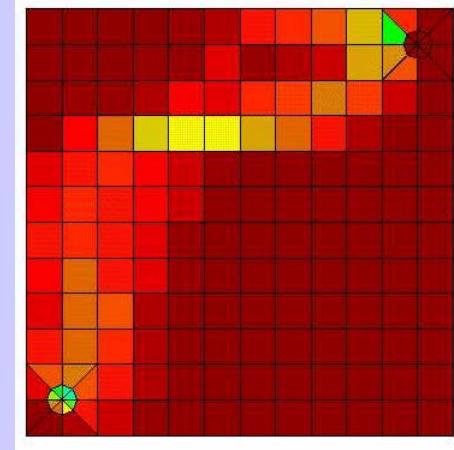
a) 60X60 ORIGINAL



b) 60X60 UPSCALED

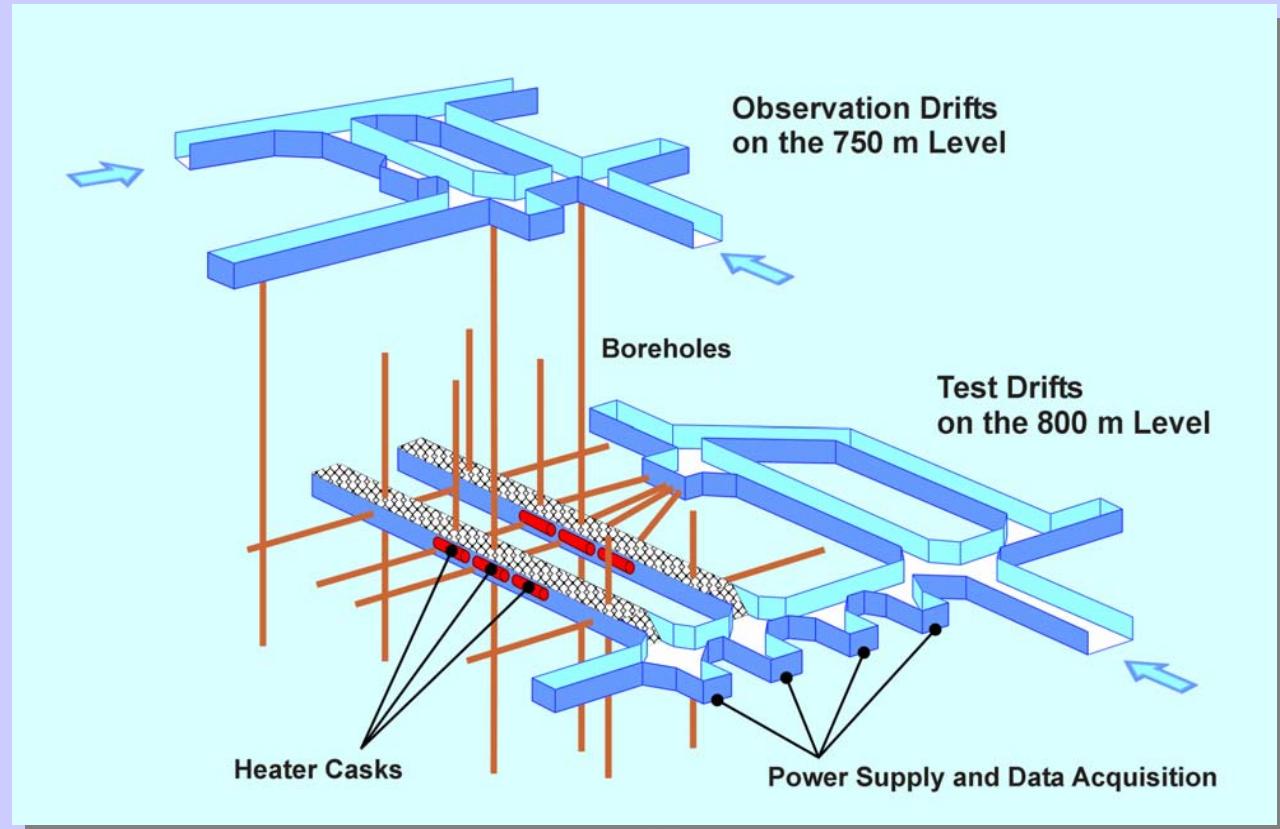
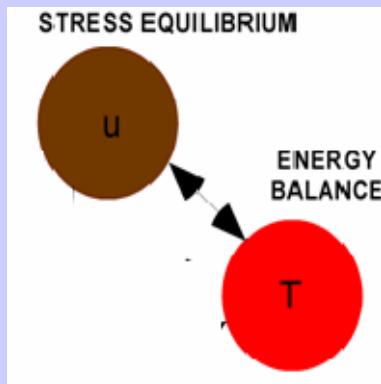


c) 12X12 UPSCALED

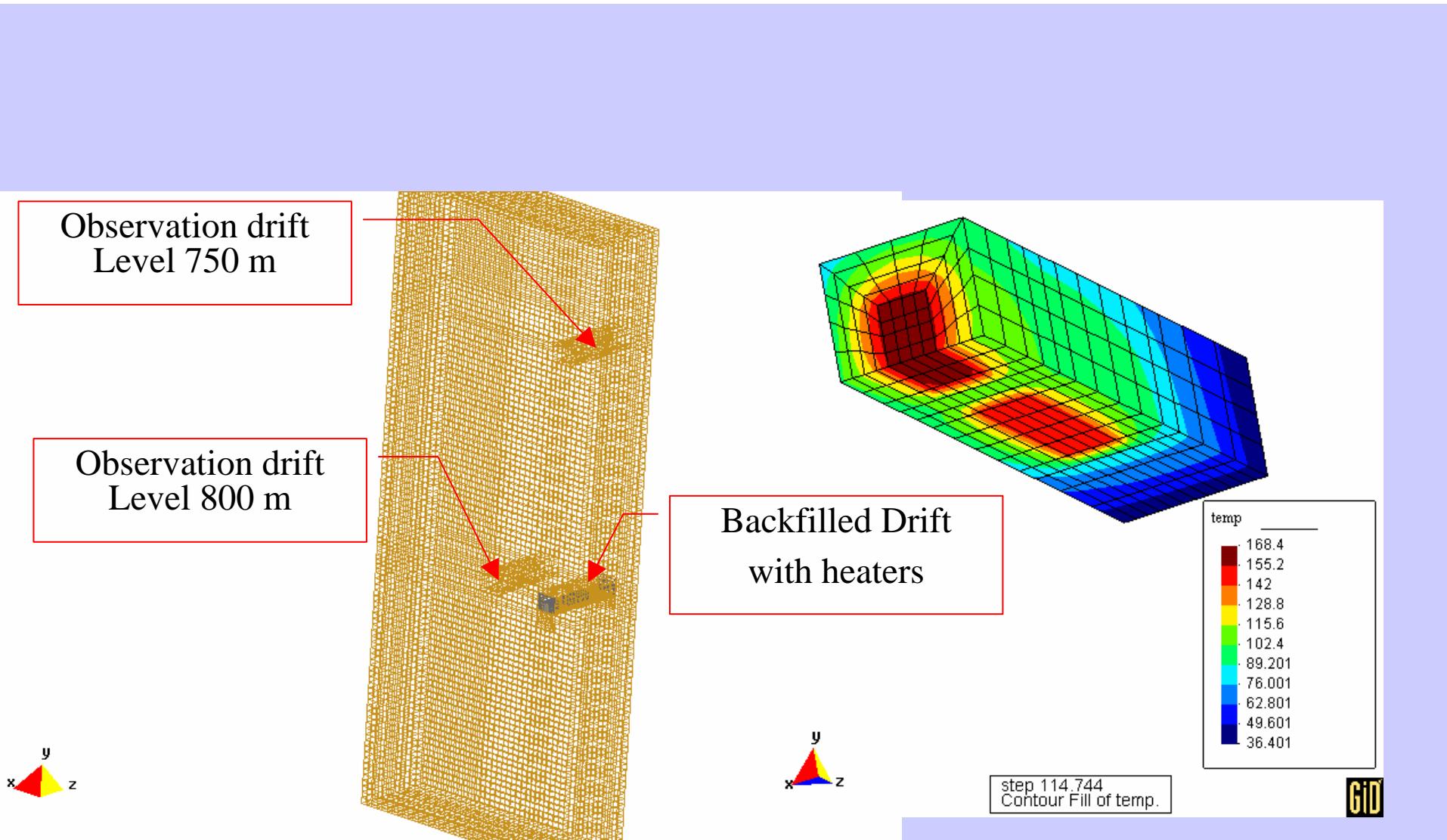


Gas fluxes and degree of saturation

TM: ROCK SALT IN SITU TEST

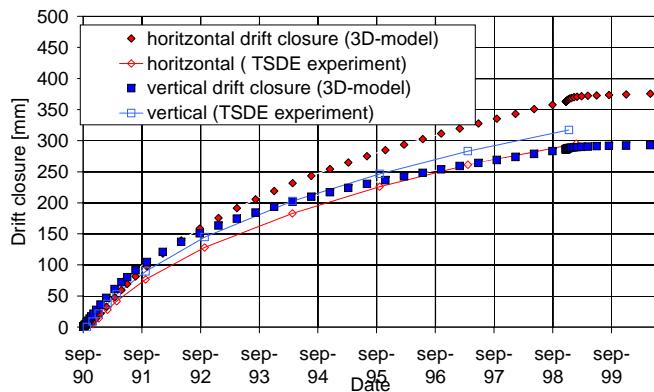


BAMBUS-I and BAMBUS-II: EC research project



Features: Tunnel + Observation drifts, $\sigma_{\text{conf}} = 12 \text{ MPa}$, Axial length: 25 m, hexaedral elements, regular mesh, 65,954 nodal points x 4 dof = 263,816 dof

7. DRIFT CLOSURE IN THE HEATED AREA
(BAMBUS II - Project : 3-DIM modelling studies)

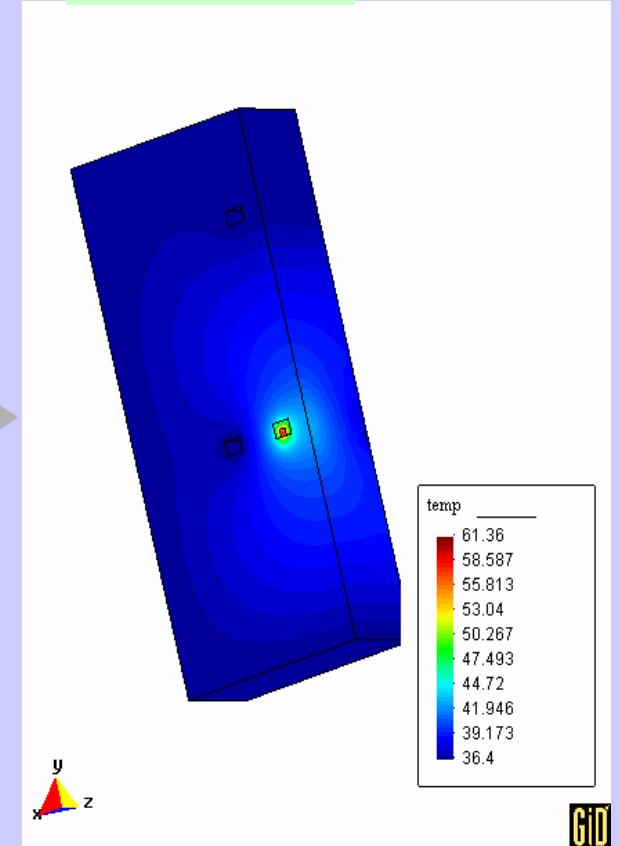
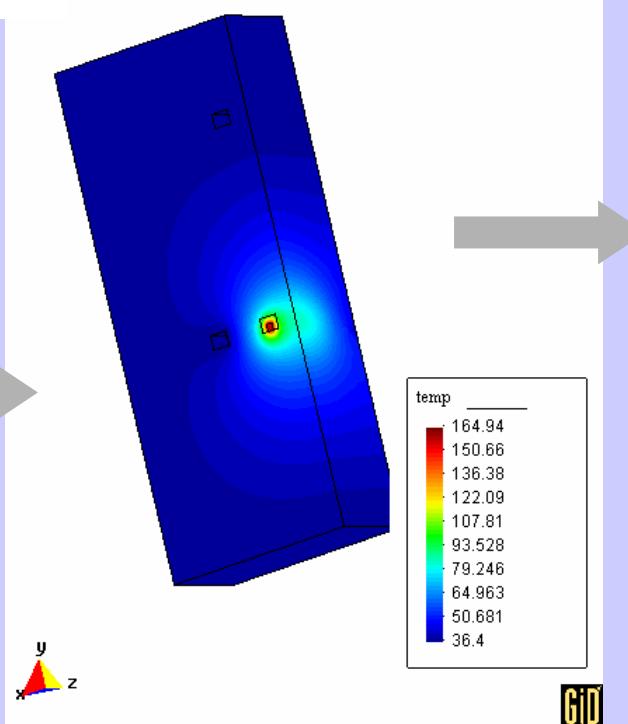
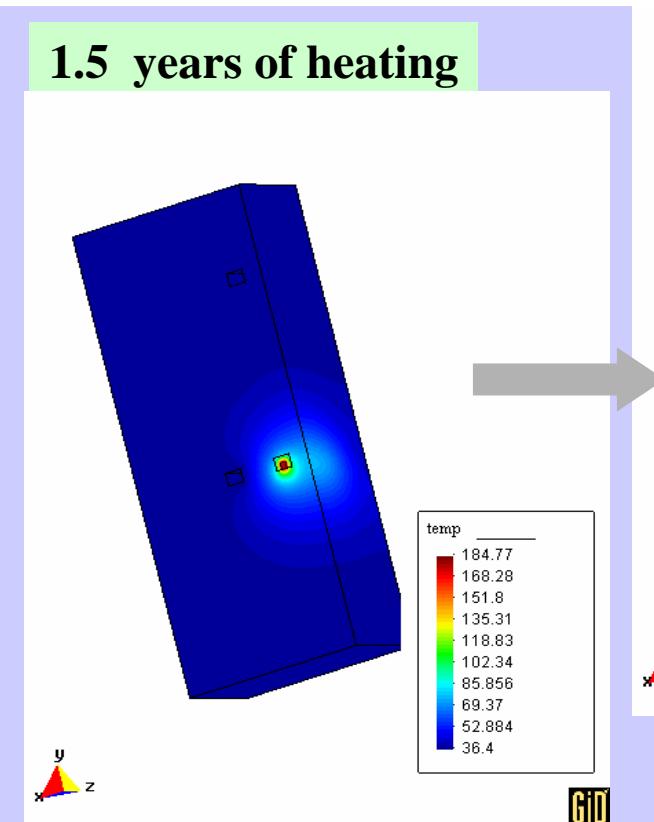


Temperature evolution (whole model)

end process

9 years of heating

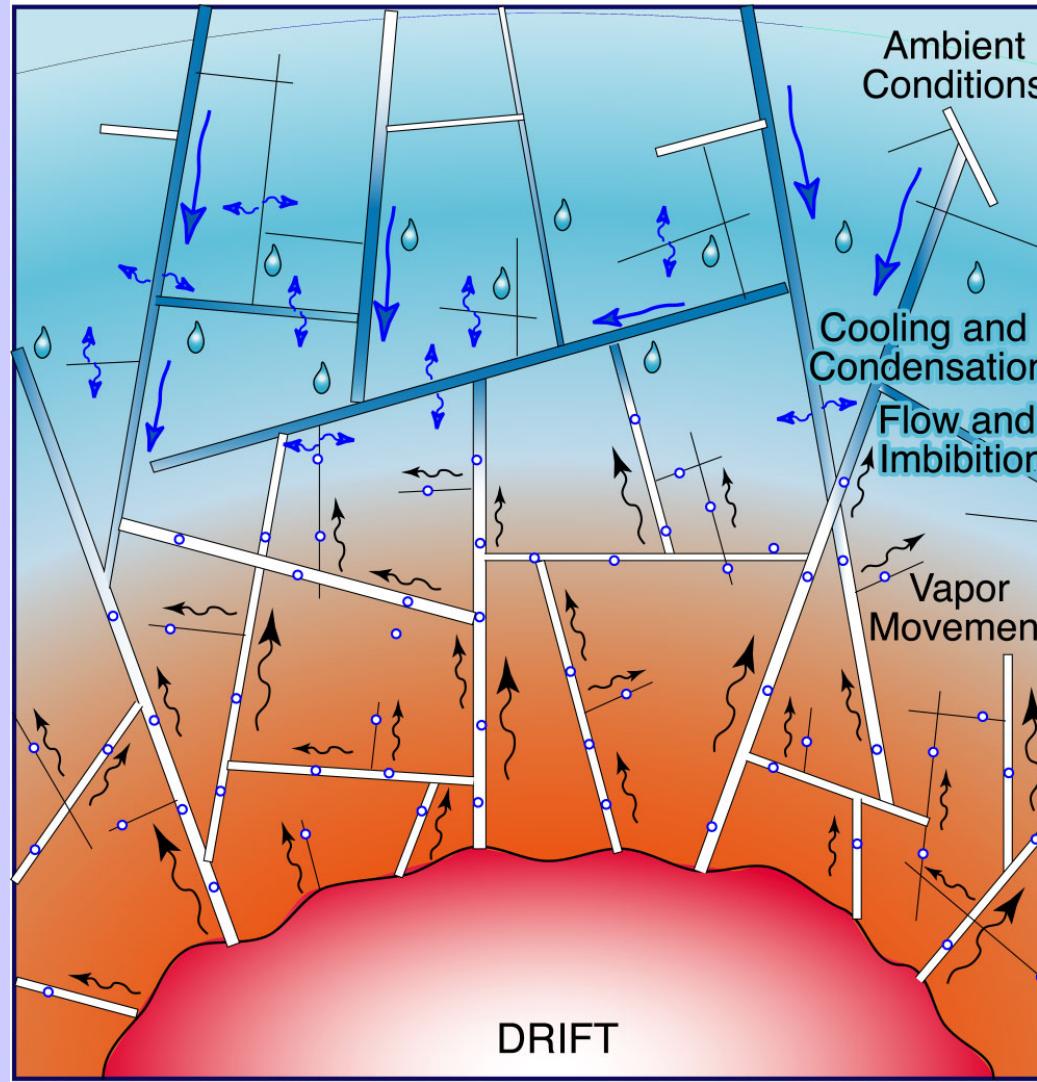
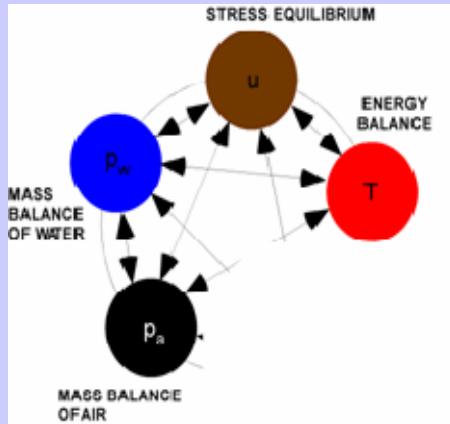
1.5 years of heating



Formulación para
problemas THM

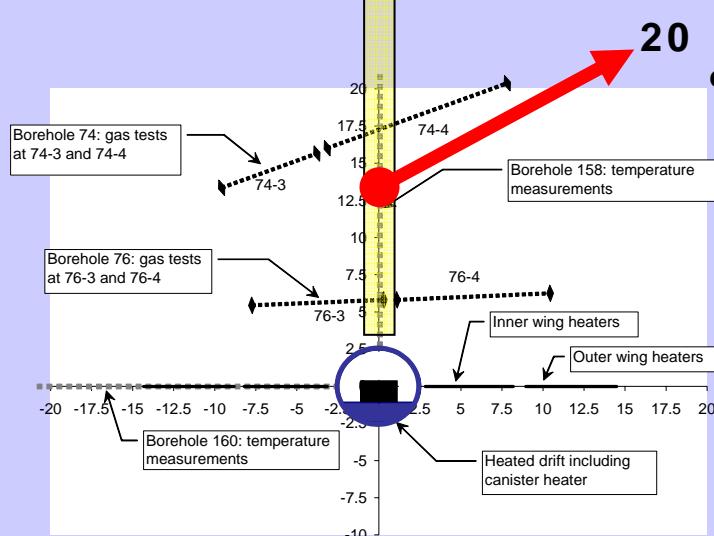
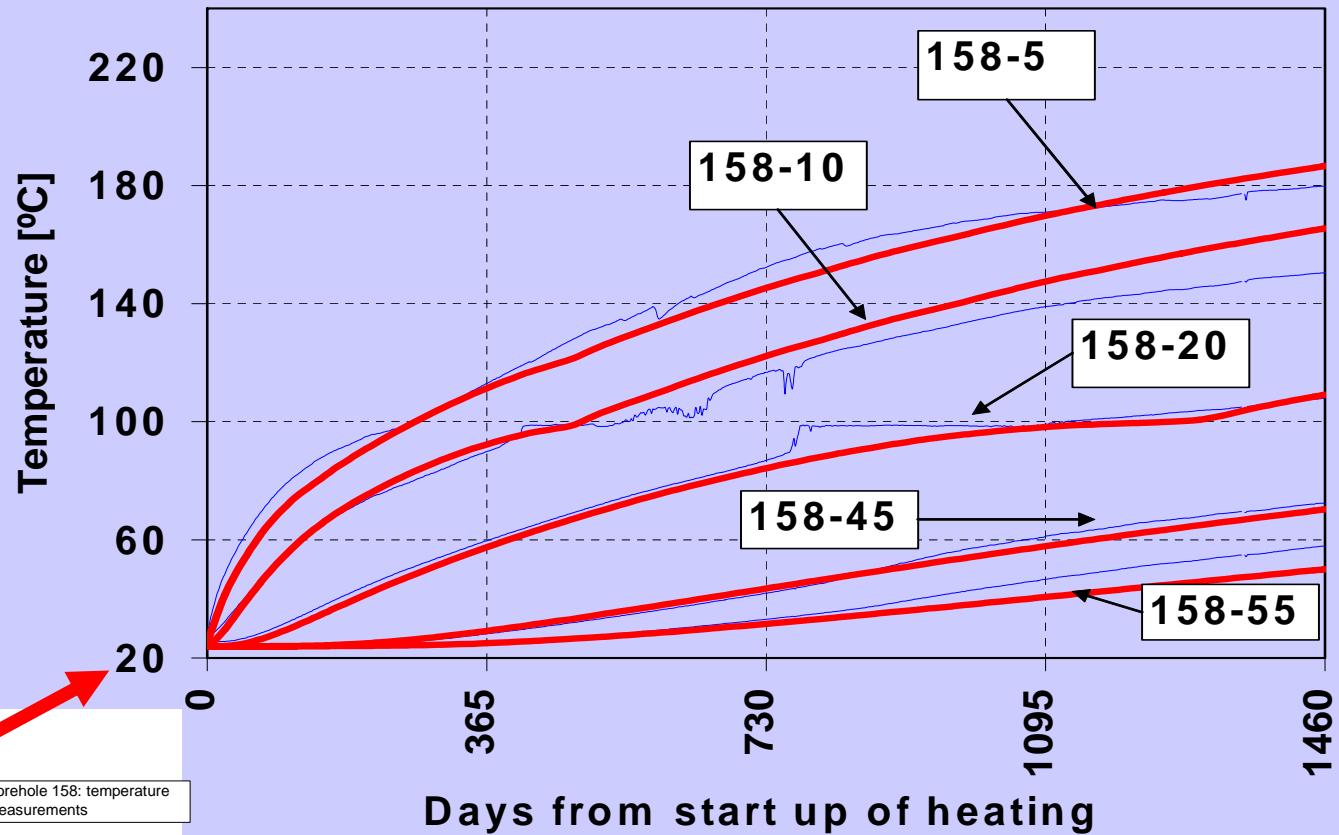
(THM) Residuos Radioactivos en Roca fracturada

□ Evaporation and condensation of water (fractured rock)



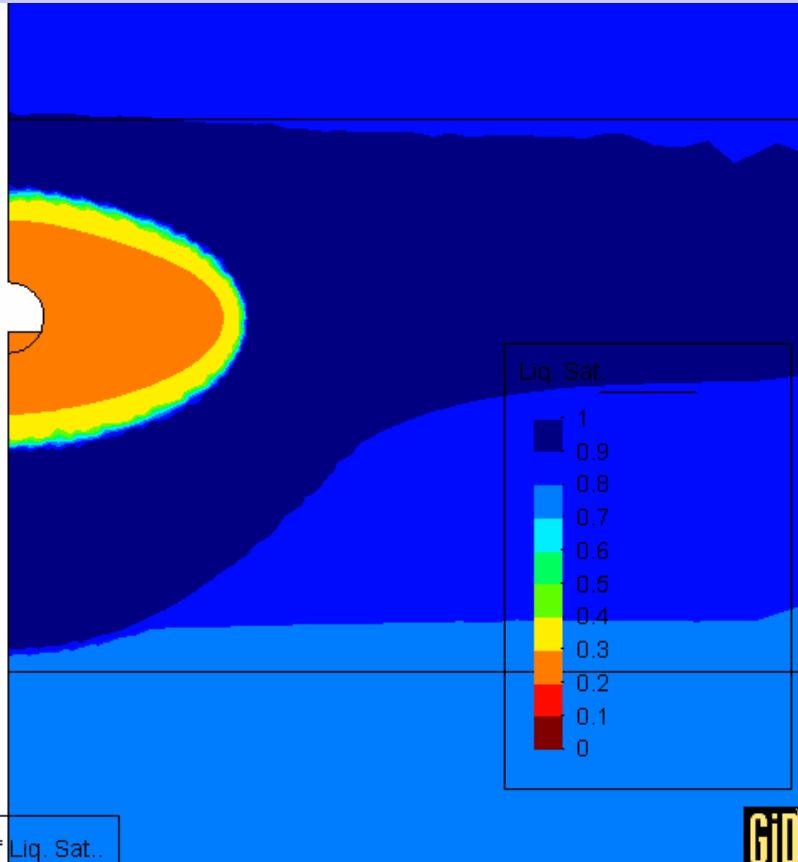
Formulación para
problemas THM

Temperaturas

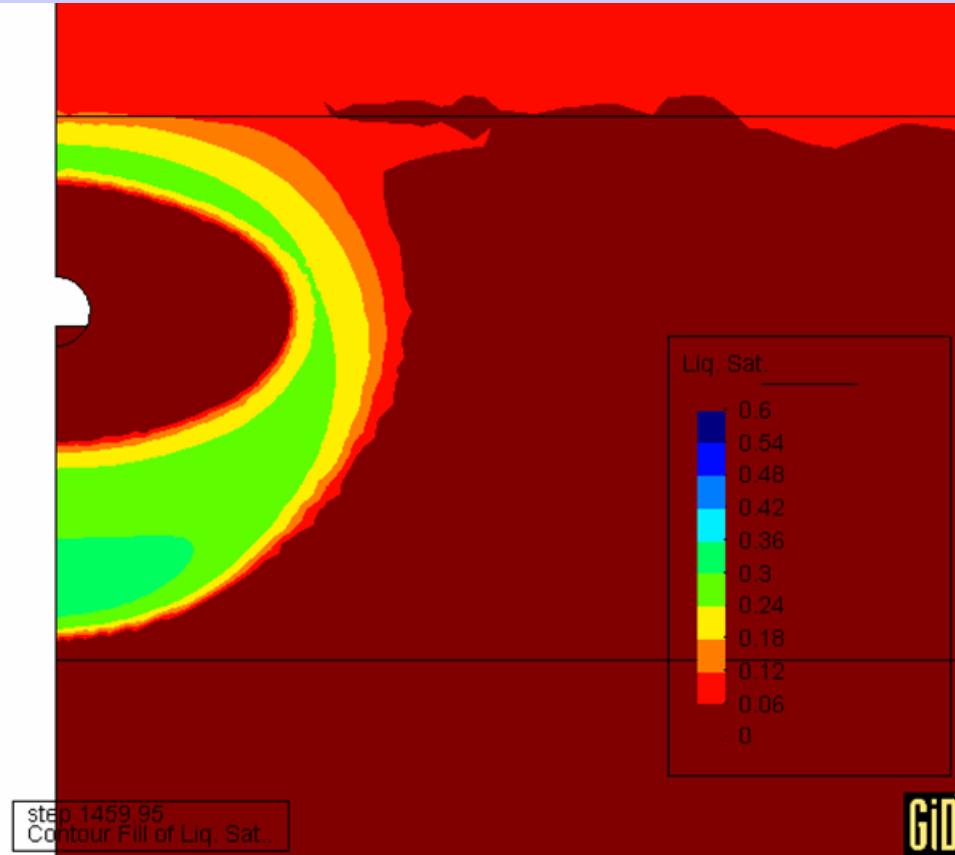


Grado de saturación en la roca y en las fracturas

Degree of saturation (after 4 years)



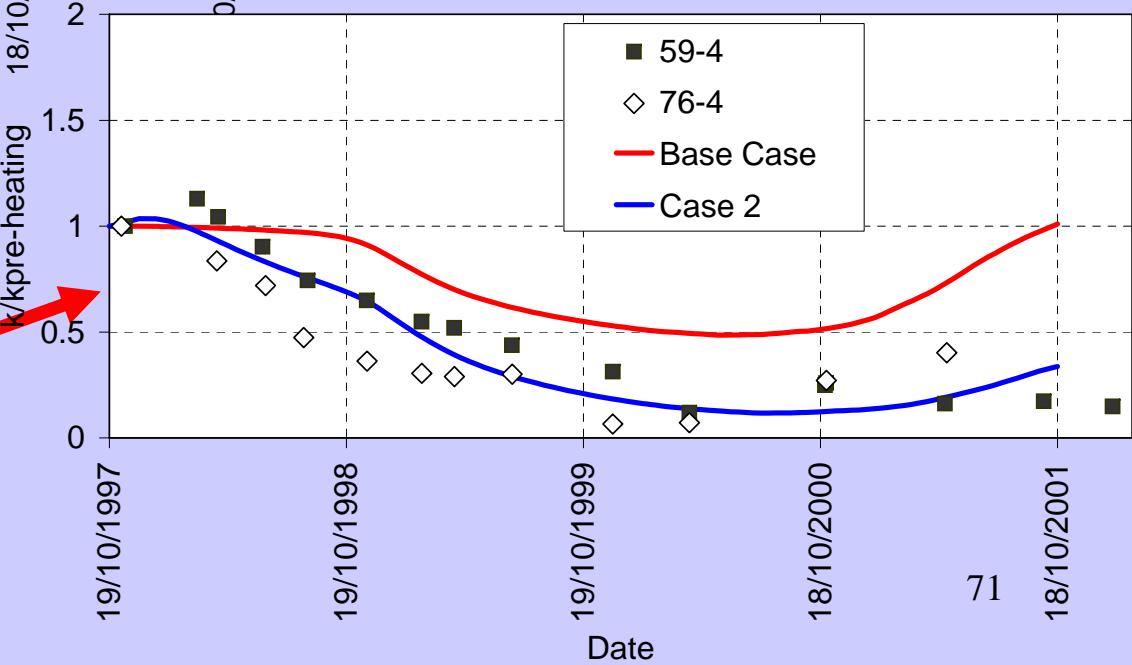
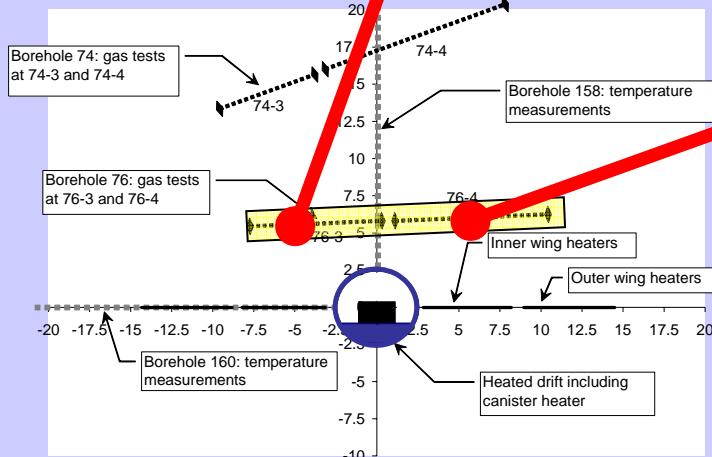
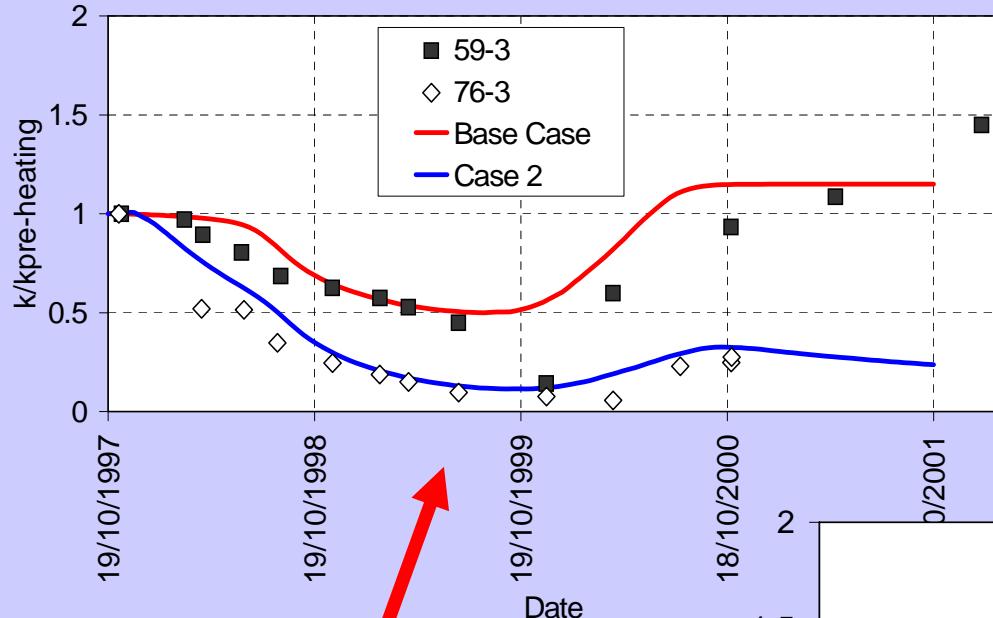
Matrix
(initial degree of saturation = 0.92)



Fracture
(initial degree of saturation = 0.05)

Case 2 analysis (thermomechanical and mechanical coupling)

Gas permeability variation near drift



Comentarios finales

- Es conveniente que se puedan elegir las ecuaciones a resolver y fenómenos a incorporar
- Las condiciones de solución numérica cambian mucho según el problema a resolver
- Todos los acoplamientos conocidos hay que incorporarlos en la formulación
- Fenómenos acoplados versus procesos acoplados